43rd Street Outfall Basin Study

Basis of Design Report Project ST510C; Work Order 22

Prepared For:
City of Tampa
Stormwater Department



Submitted To: Southwest Florida Water Management District



Prepared By:



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1. INTRODUCTION

1.1. Background

Flooding problems have persisted along a segment of the 43rd Street Outfall ditch from between Columbia Street to the 43rd Street crossing. The City of Tampa has indicated the frequency and severity of the flooding problems have increased since widening of I-4 reconstruction was completed in 2004. The Hillsborough Area Regional Transit (HART) Authority has documented structural and site flooding at the headquarters facility on 21st Avenue, to the north of Columbia Street. Both HART and the City have documented roadway overtoppings at 10th Avenue and 43rd Street. Past flooding problems are evident at the 43rd Street crossing with a berm that has been constructed along the ditch bank by an adjacent property owner.

1.2. Purpose and Scope

The City of Tampa and HART are cooperatively funding this study to; 1) analyze the flooding problems within the study area, 2) develop and compare preliminary alternatives to improve flood protection, 3) make a recommendation to improve flood protection, and 4) to incorporate water quality improvements as may be beneficial in seeking cooperative funding from the SWFWMD for design and construction phases. The City has authorized DRMP, Inc. to proceed with this study with the scope of work outlined below. The surface water model and GIS development for this study has generally been conducted in accordance with the *City of Tampa GIS Stormwater Model Parameterization Guide* (City GIS Guide), dated December 2011. Alternatives and recommendations in this report are preliminary in nature with additional survey and data collection necessary for the final design. Note that all elevation data for this study is reported in the NAVD88 vertical datum.

Task 1: Data Collection

This task involved collection and review of all data relevant to the flooding problems and developing a surface water model for the entire 43rd Street Outfall drainage basin. Data collected generally included; flooding documentation, available GIS layers, LiDAR terrain, City drainage and infrastructure information, Environmental Resource Permit (ERP) files at SWFWMD, and FDOT roadway projects. Supplemental field survey of existing culverts and ditch cross-sections was also conducted for this study with assistance from Polaris Associates, Inc. under separate authorization with the City.

Task 2: Develop Existing Conditions Model

This task involved developing a surface water model using XP-SWMM for the drainage basin contributing to the 43rd Street Outfall system. The model has been verified for accuracy by comparing peak stage results with observed and documented flooding conditions. A critical duration analysis was conducted of 5-year storms with the following durations: 1-hour, 2-hour, 4-hour, 8-hour, and 24-hour. The duration exhibiting the highest stages and flows in the focal area of the study was used for flooding and alternatives analyses. The 25-year, 24-hour storm was also simulated for design and permitting considerations of the alternatives analysis.

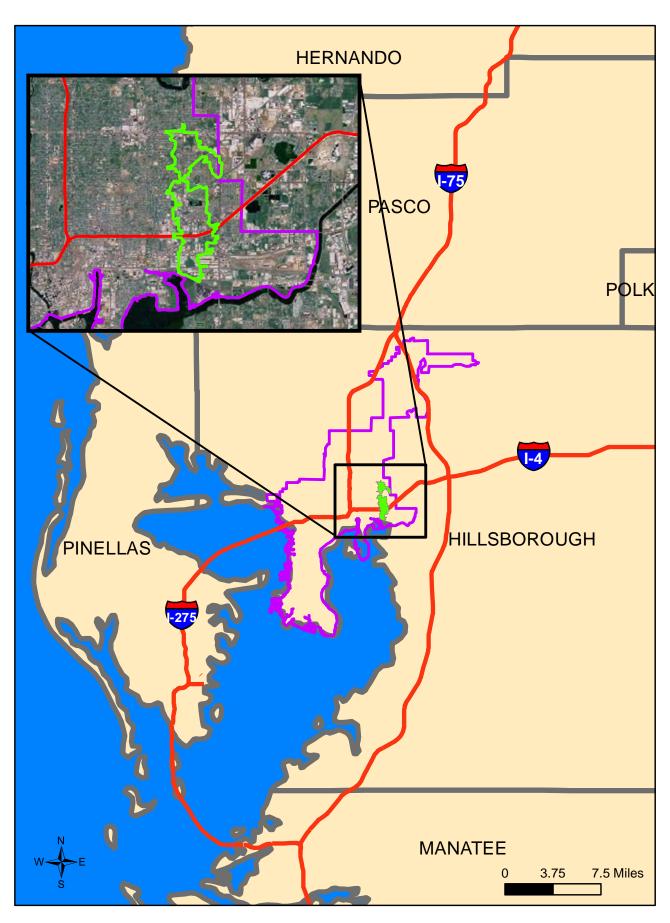
Task 3: Alternatives Analysis

This task involved preliminary analysis of feasible alternatives to address flooding problems in the focal area of the study from the HART facility southward to the 43rd Street crossing. Flood protection alternatives have been preliminarily designed to meet the level of service protection for the 5-year critical storm event. Alternatives considered include culvert and ditch conveyance upgrades, flow diversion, detention ponds, or any combination thereof. Potential utility conflicts have been researched and assessed for level of impacts for each alternative, with a summary table included in this report. Preliminary construction and property costs were tabulated for the alternatives. Water quality improvements werel also be incorporated into the alternatives where feasible. This final Basis of Design Report (BODR) documents the study and the recommendations, for ultimate submittal to the SWFWMD. Additionally, a GIS directory with all project files and model development has been submitted separately as described in the City GIS Guide.

2. PROJECT APPROACH

2.1. General Description of Study Area

The study area is the entire drainage basin contributing to the drainage system known by the City of Tampa as the 43rd Street Outfall system. This drainage basin comprises approximately 1,118 acres of mostly urban lands. Figure 1 on the following page shows location of the drainage basin for the 43rd Street Outfall.



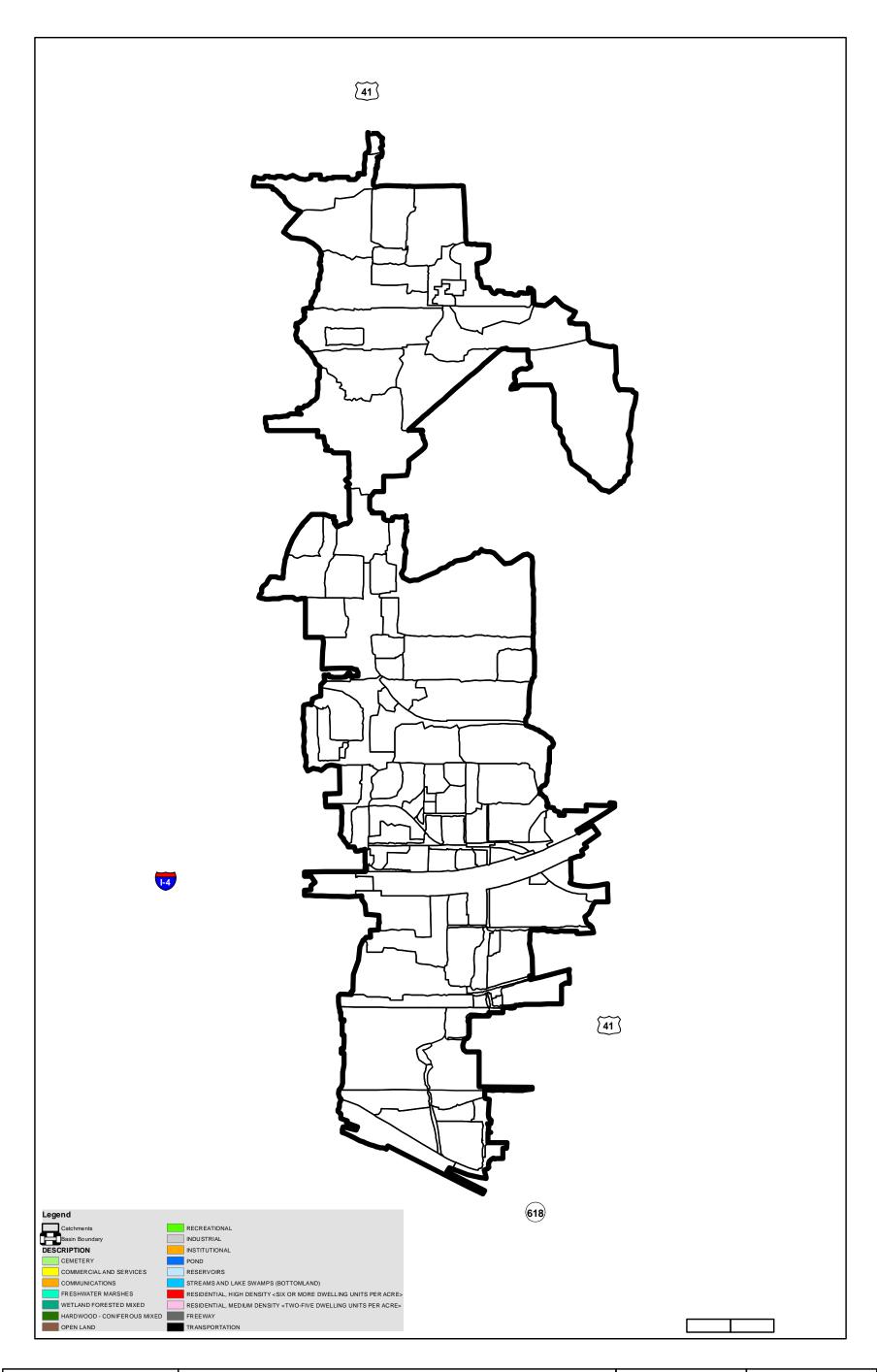


A land use GIS layer has been developed for this study originating from SWFWMD in the FLUCCS format. Some land use classifications have been adjusted for this study for modeling purposes. The land use layer is depicted on Figure 2 on the following page. The top two land uses include Commercial and Services at 38% and High Density Residential at 26%. Table 1 below is a breakdown of the land use types with acreages, percentages of the study area, and estimated percent imperviousness by land use type.

Table 1 – Land Uses

			Estimated
		Percentage	Percentage of
	Area	of Total	Impervious Area
Land Use Classification	(acres)	Area (%)	(%)
COMMERCIAL AND SERVICES	422.9	37.8	70
RESIDENTIAL, HIGH DENSITY <six or<="" td=""><td></td><td></td><td></td></six>			
MORE DWELLING UNITS PER ACRE>	288.2	25.8	33
CEMETERY	85.9	7.7	13
FREEWAY	65.8	5.9	84
INDUSTRIAL	64.5	5.8	38
INSTITUTIONAL	60.5	5.4	54
TRANSPORTATION	35.7	3.2	91
HARDWOOD - CONIFEROUS MIXED	30.3	2.7	0
POND	29.7	2.7	71
RECREATIONAL	12.1	1.1	11
RESIDENTIAL, MEDIUM DENSITY <two-< td=""><td></td><td></td><td></td></two-<>			
FIVE DWELLING UNITS PER ACRE>	7.0	0.6	36
OPEN LAND	6.6	0.6	0.0
STREAMS AND LAKE SWAMPS			
(BOTTOMLAND)	3.6	0.3	0.0
FRESHWATER MARSHES	3.5	0.3	0.0
RESERVOIRS	0.8	0.1	0.0
COMMUNICATIONS	0.7	0.1	0.0
WETLAND FORESTED MIXED	0.6	0.1	0.0

Totals 1,118.4 100.0



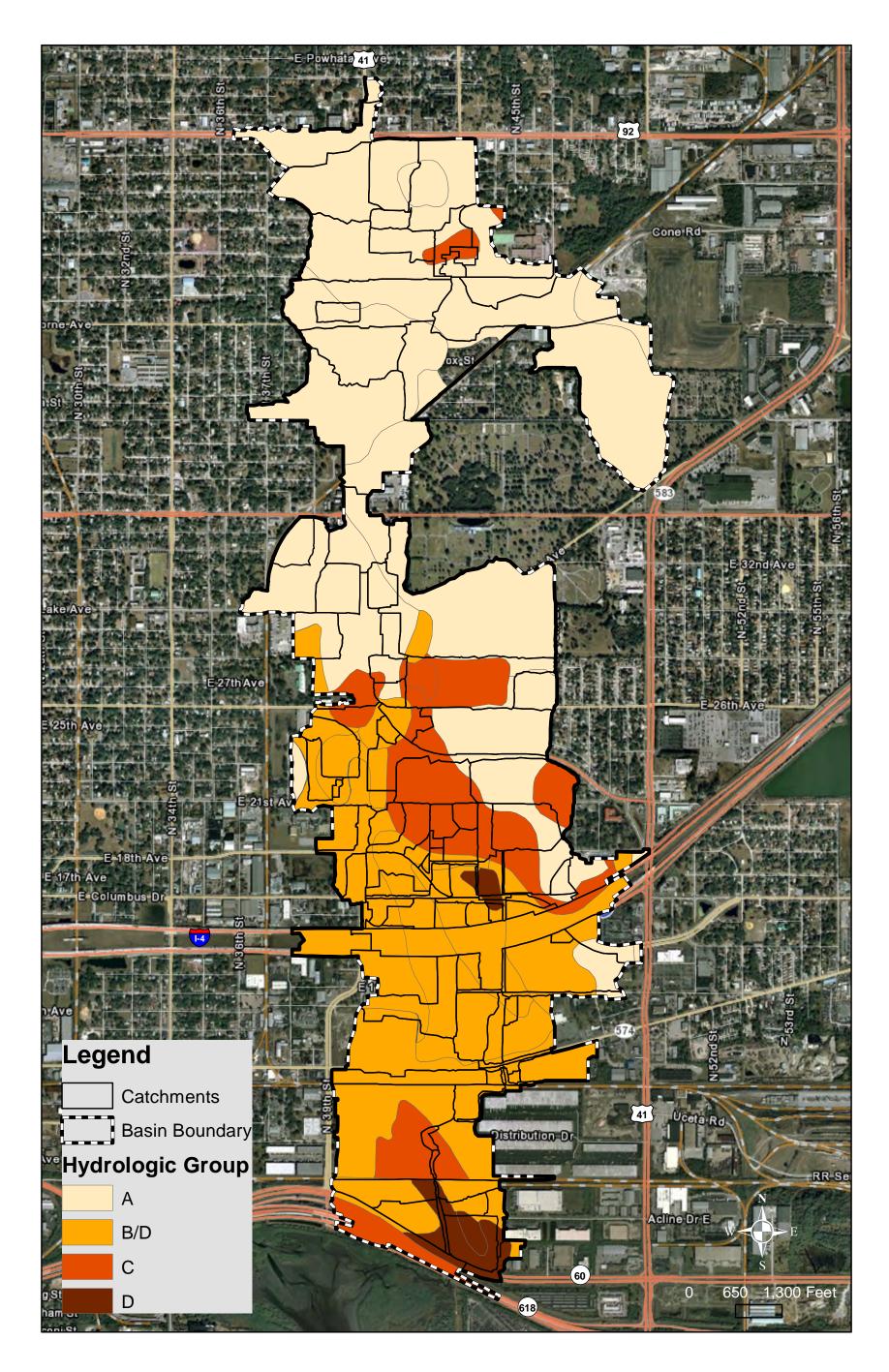


The soils within the study area vary from well-drained sandy soils, characterized as hydrologic soil group (HSG) type A, in the northern and central parts at higher elevations to poorly-drained silty sands in the southern parts of the study area, characterized as HSG types C, B/D, and D. Soils classified as B/D are characterized as type D in a native and undrained state, however, they are considered B in urban areas with extensive and long-established drainage systems such as this study area. Therefore, B/D soils in this study area are characterized as type B. Figure 3 on the following page depicts the HSG soil types for the study area based on a soils GIS layer from SWFWMD. Table 2 below is a breakdown of the HSG soil types with acreages and percentages of the study area.

Table 2 – Soils

Hydrologic Soil Group	Area	Percentage of	
(HSG)	(acres)	Total Area (%)	
А	575.9	51.5	
B (B/D)	363.5	32.5	
С	151.7	13.6	
D	27.3	2.4	

Totals 1118.4 100.0





2.2. Data Collection

Data collection was conducted at the onset of the study, with the following general description of relevant data:

- HART and City documentation of flooding on July 9, 2011
- HART site map
- City of Tampa GIS stormwater inventory (including drainage structures, piping, and channels)
- GIS data including LiDAR-based terrain, land use, soils, parcels, roadways, wetlands
- City of Tampa Drainage Atlas (historic hard copy maps)
- Environmental Resource Permit (ERP) files from SWFWMD
- FDOT roadway plans, drainage maps, and supporting calculations
- City of Tampa roadway and stormwater retrofit projects
- Surface water models of adjacent basins
- Field reviews to confirm drainage patterns and characteristics
- Survey of select channel cross-sections and culverts conducted by Polaris for this study

LiDAR data was obtained from SWFWMD based on flights taken in 2007 in the NAVD 88 vertical datum. This LiDAR data was developed into GIS terrain for extensive use in this study in developing the surface water model. Following are some of the major roadway and project construction plan sets that were utilized in developing the model for this study.

- Final Roadway Plans for reconstruction of SR 585A and 45 (N 40th Street) dated 1966
- Melburne Boulevard Pond Retrofit Project, Record Drawings dated October 3, 2003
- Final Roadway Plans for widening of SR 400 (I-4) dated 2003
- Permitted Roadway Plans for Lee Roy Selmon Crosstown Expressway dated 2002

Two studies of adjacent basins are available on the east side of this study area as described below:

- East Lake Watershed Management Plan Update, prepared for Hillsborough County, study conducted by Ayres Associates completed in January 2007
- Southern Portion of 29th Street Outfall Drainage Study, prepared for City of Tampa, study conducted by Bayside Engineering, Inc in August 1997 (with future model update and retrofit design proposed by Kimley-Horn and Associates)

All data historical or paper sources utilized in developing the surface water model are documented in the GIS modeling directory by hyperlink of the model features to "pdf"

documents of the relevant plan sheets with "clouding" of the specific data used. This methodology for documenting the model development (model backup) follows requirements set by the *City of Tampa GIS Stormwater Model Parameterization Guide* (or City GIS Guide).

2.3. Drainage Basin Characterization

The drainage basin for the 43rd Street Outfall is a linearly shaped basin approximately 3.2 miles long in the north-south direction with the upper 2.2 miles comprised of the storm sewer system for SR 599/569, a major arterial FDOT roadway also known as US 41 and locally known as 40th Street. The northern reaches of the system exist as an inter-connect with the storm sewer of US 92, or Hillsborough Avenue, that runs in the east-west direction. At the 40th Street and Hillsborough Avenue intersection, a 30-inch pipe is directed eastward and a 24-inch pipe is directed southward, so a smaller share of the flows is expected to flow south through the study area, with majority of flow traveling east out of the study area.

For the upper two-thirds of the drainage system length, the 40th Street storm sewer is the primary component, increasing in size to a 4-foot by 11-foot box culvert at the divergence from the roadway. Invert elevations along the system start at 46.1 ft, NAVD and fall 22.6 feet at a 0.20% slope to 23.5 ft, NAVD at the divergence from 40th Street. Along this route there are a number of connections from private stormwater pond outfalls serving commercial and residential developments. There are also several side street storm sewer systems that converge with the 40th Street system; notably at East Chelsea Street, East Lake Avenue, and East 28th Avenue. An important component of this system is a large stormwater treatment pond that was the subject of a major renovation in 2002 known as the Melburne Boulevard Pond Retrofit Project. This site was previously an FDOT borrow pit that had been connected to the 40th Street system as an off-line attenuation pond. The retrofit involved re-piping the system so that all flows go through the pond that has been broken up into treatment cells with specific functions being settling particles, treatment with wetlands plants, and residence time with open water permanent pool. Flows from this pond flow back into the 40th Street storm sewer via a 8foot long concrete weir located within a structure in the road right-of-way.

The primary drainage system exits the 40th Street right-of-way at Columbus Drive and traverses eastward via ditch with numerous large box culverts at driveways. This segment of the system crosses from the north side of Columbus Drive to the south side at a Y-intersection with 19th Avenue. Additionally, several local drainage systems converge in this area including a segment of I-4, some highly impervious commercial sites, and a system from the HART headquarters facility. The HART system is part of the focal areas of this study. The HART system originates from the north at the 21st Avenue storm sewer that collects drainage from a residential area approximately 36 acres in size. This system enters the HART facility as a 29x45-inch storm pipe. The HART facility is drained with several ditch bottom inlets along the pipe system and surface flows from graded paved areas into adjacent ditching. This pipe is discharged into a ditch with an adjoining ditch from the east, that flows southward to the Y intersection at 19th Avenue

and Columbus Drive. It should be noted that residential and commercial areas drain into the HART collection system with no attenuation as these areas were developed prior to current stormwater rules that require attenuation and treatment. Other surrounding school, park, and commercial sites appear to have been constructed more recently with stormwater ponds connecting to the primary drainage system downstream of the HART system. Invert elevations along this part of the system fall 0.7 feet at a 0.03% slope from 23.5 ft, NAVD to 22.8 ft, NAVD at the headwall under I-4.

The 43rd Street outfall drainage system crosses the I-4 corridor through a 408 foot long 4-foot by 12-foot box culvert and travels generally southward as a ditch with box culvert crossings at roads including 10th Avenue, SR 574 (7th Avenue), and two crossings of railroad tracks. This portion of the system is another focal area of the study. Through this segment of the system, the ditch is described with a channel cross section of 15-foot wide bottom with steep banks and flows confined within the banks except at overtopping of roadway crossings. Flows enter the ditch along this segment from off-site residential and industrial/commercial areas, some with on-site stormwater ponds. The ditch system makes several jogs westward and crosses under 43rd Street through dual 36-inch culverts. This pipe crossing causes a constriction of flow in the ditch system with water backing up in the ditch and flowing across private properties. There is evidence of this flooding with protective measures that one property owner has taken by building an earthen berm along the property line. Invert elevations along this part of the system fall 9.1 feet at a 0.27% slope from 22.8 ft, NAVD to 13.7 ft, NAVD at the headwall of the 43rd Street crossing.

The southern portion of the 43rd Street outfall system is a ditch with three more sets of railroad crossings, the southern-most being a wooden bridge on piles. The ditch traverses through a sizeable undeveloped area that serves as overbank storage during high flows. Flows contribute to this segment from large industrial and commercial sites, with some of the smaller ones including stormwater ponds. The system leaves the study area at the crossing under SR 60 and the Lee Roy Selmon Expressway (SR 618) that is a 364 foot long 8-foot by 7-foot box culvert. Invert elevations along this part of the system fall 16.4 feet at a 6.36% slope from 13.7 ft, NAVD to -2.7 ft, NAVD at the headwall of the SR 60 crossing. On the south side of this crossing the system discharges into the northern-most reach of McKay Bay in the estuarine part of Tampa Bay.

2.4. Flooding Conditions

Flooding problems are known to occur generally in the southern half of the drainage basin. Several documented flooding areas that are listed and further described below are the focus area of this study:

- 1. HART Authority headquarters facility on 21st Avenue
- 2. South side of Columbus Drive, 43rd Street outfall ditch
- 3. South side of I-4, 43rd Street outfall ditch
- 4. 10th Avenue, 43rd Street outfall ditch
- 5. 7th Avenue (SR 574), 43rd Street outfall ditch
- 6. 43rd Street, outfall ditch

These locations of witnessed flooding are also documented in the accompanying GIS database as "flooding points" with accompanying pictures and videos taken by HART and City staff after a storm event on July 8, 2011. Rainfall records were reviewed from both SWFWMD and NOAA archives for all recording stations in the vicinity of the study area. The Tampa International Airport (NOAA site) is 7 miles to the west with recorded 3.7 inches of rainfall over a 11-hour period. Two stations at SWFWMD water control stations are 2.4 and 3.0 miles to the east with recordings of 4.7 and 5.0 inches over 6-hour periods.

The photos and video of the flooding at the HART facility shows flooding of service bays on the west side within the main building at about 6-inches deep and flowing south. Also, the southeastern part of the parking area is flooded up to about the middle of the tires on the buses. Video taken at 10th Avenue shows flooding over the roadway for an estimated distance of 420 feet from the culvert crossing westward to the stop sign at 43rd Street. Video taken at 7th Avenue does not show roadway flooding, however, it does show high flow velocity and falling water surface profile on the downstream side of the culvert crossing. Video taken at 43rd Avenue shows water sheet flowing across private property toward the large wooded area to the south where the ditch re-emerges. Still images extracted from the videos of documented flooding with locations are also depicted in Figure 4 on the following page.

Scott Garth (DRMP) field reviewed the HART facilities and adjacent properties on June 24, 2012 (Tropical Storm Debby) and observed no flooding, leading to the belief that flooding in this basin appears to be associated with short intense rainfall events.





2.5. Model Development

A surface water model has been developed for this study using the computer program XP-SWMM. The underlying program is the Environmental Protection Agency (EPA) Storm Water Management Model (SWMM) with utilization of two components, or blocks, for this study. Stormwater runoff hydrographs are developed using the RUNOFF block with a collection of sub-catchment, or basins, and rainfall distributions. The hydrographs are dynamically routed through the hydraulic junction-link network using the EXTRAN block.

2.5.1 GIS Database Framework

A GIS database has been developed for this study following the guidelines in the *City of Tampa GIS Stormwater Model Parameterization Guide* (or City GIS Guide). A junction-link network and catchments are included in this GIS database in support of the XP-SWMM model framework with all relevant hydraulic and hydrologic information included in attribute tables. Soils and land use layers are included as utilized for developing the hydrology of the basins. Sampling of impervious area by land use was conducted for the purposes of developing hydrology parameters with a supporting GIS layer. Model backup information is included as "pdf" documents that are hyperlinked in GIS with red "clouding" directing attention to information utilized in building the model. Times of concentration flow paths are included with supporting hydraulic data in an attribute table. Topography is included with a terrain and 0.5 foot contours based in LiDAR. Other features are also included such as the vertical datum conversion, flooding points, government-owned parcels, stormwater ponds, and the Tampa stormwater inventory.

2.5.2 Vertical Datum

All deliverables for this study are based on the **NAVD 88 vertical datum**. The main data sources for the model are referenced to the NAVD 88 datum including the terrain and survey conducted for this study. However, a number of other supporting documents for building the model are referenced to the NGVD 29 vertical datum. The following conversion factor was used as necessary for this study:

Elevation in NAVD 88 = Elevation in NGVD 29 – 0.86 feet

This conversion factor is the average elevation difference between these two datums based on three points spaced out within the study area as determined from the CORPSCON program supported by the Army Corps of Engineers. The three points and supporting information for the datum conversion are included as a GIS layer.

2.5.3 Hydrology Methodology

The SCS Runoff Curve Number (CN) method has been chosen as the hydrology method for this study. Other methods such as Green-Ampt or Horton were considered and not chosen due to the high percentage of poorly-drained soils, especially in the focus area of the study. To meet City GIS Guide criteria, both DCIA and nDCIA impervious areas were estimated for each catchment based on GIS intersection of catchments with the land use layer. Impervious areas were estimated by choosing samples of typical development units within each land use category and delineating the amount of DCIA and nDCIA within each sample. Percentages for each land use category were calculated and utilized in the GIS intersection with catchments. Note that for modeling purposes, DCIA and nDCIA were combined into a total impervious area value. CN's were calculated for each catchment for the pervious area only. These composite CN values were developed using the CN values for the land use category, open space, with fair hydrologic condition and varying hydrologic soil group (HSG) soil types as listed in Table 2-2a of the TR-55 manual titled *Urban Hydrology for Small Watersheds* by the Soil Conservation Service (SCS), dated June 1986. Additionally, for any B/D HSG types the B values were used due to extensive and historic ditching throughout these parts of the study area. The impervious area sampling and CN calculations are included as GIS layers and supporting spreadsheets in the electronic delivery of this study.

Times of concentration (Tc's) have been developed for each catchment using TR-55 methodology. The upstream flow regime is considered sheet flow up to a maximum of 300 feet in length and calculated using the Manning's kinematic solution. The next downstream flow regime may include shallow concentrated flow over either pervious or impervious surface with the average velocity determined using the formulas from Figure 3-1 of the TR-55 manual. In some cases, other flow regimes were used including channel, gutter, pipe, and/or channel flow with assumed velocities of 1.0, 1.5, and 5.0 fps, respectively. The T'c flow paths are included as a GIS layer and the supporting spreadsheet for calculating the T'c values is included in the electronic delivery of this study.

Catchment delineations were performed by digitizing divides primarily based on the terrain. Ditching and piping collection systems were also considered in the delineations based on the City stormwater GIS inventory and various other plans and documents as included in the GIS database. A separate catchment has been delineated for any stormwater pond greater than 0.5 acres, and some smaller ponds were also included. The primary 43rd Street ditch system has been separated out with catchments based on criteria in the City GIS Guide to ensure that overbank storage is counted separately from the channel. Also based on direction from the City in the project scope, a separate catchment is delineated for any collection system with 36-inch or larger sized piping. There are a total of 121 catchments for this model, ranging in size from 0.18 acres to 81.27 acres.

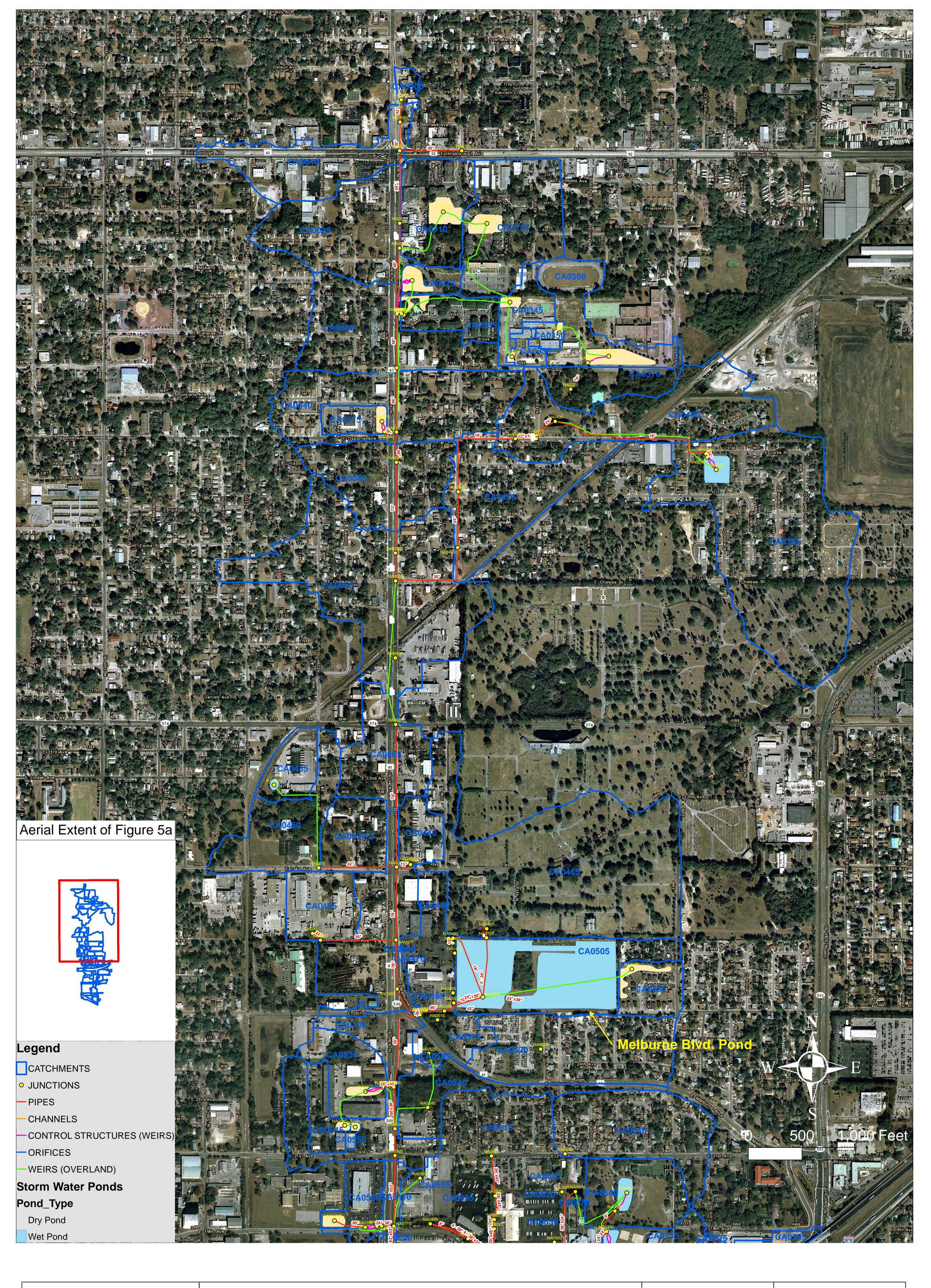
This methodology was discussed with the City and SWFWMD in a meeting on 11-16-2012 and was found to be acceptable.

2.5.4 Hydraulics

A junction-link network is set up in the XP-SWMM model to simulate stormwater routing through the study area. The northern half of the study area is composed primarily of the 40th Street storm sewer system that is modeled as a series of pipe links with junctions at drainage structures. The link-junction framework is a simplification of the physical system with junctions at pipe confluences and changes in pipe size. The southern half of the study area is composed primarily of channel and culvert segments along the 43rd Street outfall ditch. Major secondary systems with piping at 36-inch diameter or larger, or stormwater pond outfall systems are modeled hydraulically and connected to the primary system. Several secondary ditch systems also tie into the 43rd Street outfall ditch in the vicinity of Columbus Drive, one of which drains the HART facility. The junctions along the primary system are generally zero-storage nodes, with storage areas accounted for in adjacent and connecting junctions. Stage-area relationships were automatically generated using GIS by intersecting the catchments with the terrain. Onetenth foot intervals were input into the model as directed in the City GIS Guide with some simplification of data where the number of allowable data points was exceeded in XP-SWMM. All junctions are inputted in the model as "sealed" so that the 2D function of the model is not utilized. Instead, overland weirs are included in the model to account for any overtopping of berms, roads, etc. that would allow flows under flooding conditions. These overland weirs were cut from the terrain, and using guidance from the program documentation are modeled in XP-SWMM as short 35-foot long channels. The exact spatial location and orientation of these overland weirs are included as a GIS layer. Stormwater pond outfalls are generally multi-faceted links that may contain a combination of weirs, orifices, and pipes. For this model using XP-SWMM, any vertical openings in structures (round or rectangular) are modeled as "side-mounted" orifices. Horizontal open tops of structures are modeled as vertical weirs. A junction is included to represent the interior part of any structure with piping to an outfall. Note that all box culverts are described in this report with the vertical dimension x horizontal dimension (H' x W').

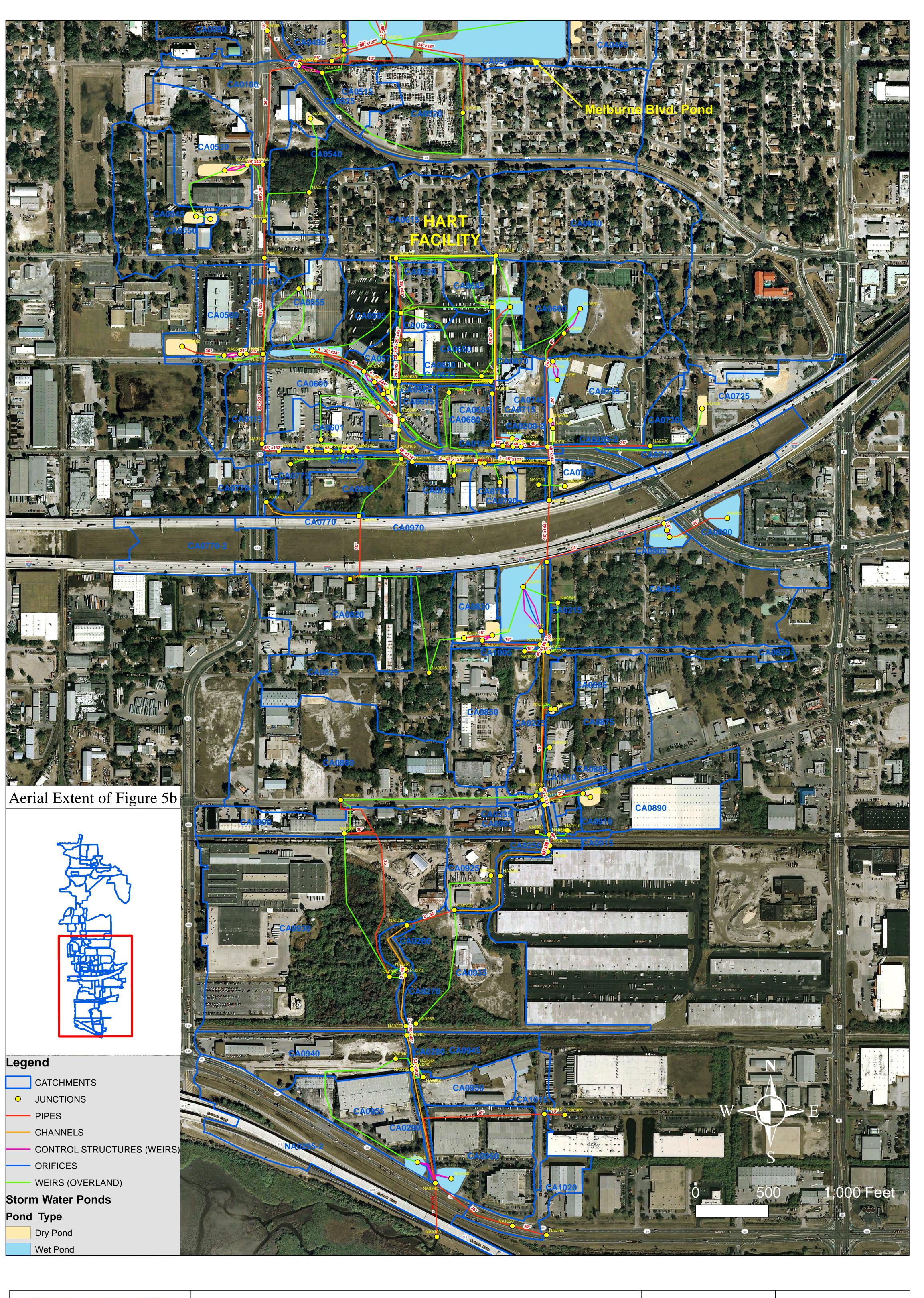
Survey was conducted for this project to accurately model the 43rd Street outfall ditch from the intersection of Columbus Drive and 19th Avenue downstream to the 43rd Street crossing. Culvert invert elevations, sizes, and types were documented at each crossing. Existing water levels and silt build-up were noted. Ditch cross-sections were surveyed at the upstream and downstream side of each culvert crossing. The collected survey data is included in the GIS database. The 43rd Street outfall ditch is modeled as a channel with irregular sections based on surveyed cross-sections and trapezoidal sections at all other locations, that are approximated based on the terrain. All other hydraulic components of the model were developed either from as-built/construction plans or other collected documentation with some approximations of elevations based on the terrain. The sources of all hydraulic data used in the model are documented in the GIS database.

The model includes the following hydraulic components: 198 junctions, 109 pipes, 10 orifices, 45 weirs, 24 channels, and 122 overland weirs.











2.5.5 Channel Base Flows

From reviewing encountered water levels and evidence of normal high conditions (staining on endwalls), it appears there is a significant base flow along the 43rd Street outfall ditch originating at the Melburne Avenue pond and increasing to the outfall culvert under SR 60. Therefore, the model was set up with base flows originating at strategic junctions along the primary ditch. Flows were determined by an iterative method of running the model and checking for stable water levels at the preferred elevations along the ditch. These initial stages were set using the "initial depth" parameter in XP-SWMM. Included below is Table 3 with the base flow values and initial depths at the selected junctions along the ditch segment with base flow.

Table 3 – Base Flow Conditions

		Initial		
		Elevation	Initial Depth in	Base Flow added
Junction Name	Location	(ft, NAVD)	Model (ft)	at Junction (cfs)
	Melburne Blvd. pond east of the		1.00 (relative to	
NA0505	waste management facility	26.94	orifice)	10
	4'x11' box culvert on north side of			
NA0165	East Columbus Dr near E 19th Ave.	25.04	2.00	34
	4'x12.2' box culvert on south side of			
NA0225	E 10th Ave.	23.70	2.80	19
	Twin 36" pipes, upstream side of			
NA0255	43rd Street crossing	15.50	2.00	26.5
	8'x7' box culvert on the north side			
NA0295	of Adamo Drive	3.70	5.96	0

2.5.6 Boundary Conditions

Boundary conditions exist at four (4) outfalls or inter-connect locations around the perimeter of the study area. The primary model outfall is located on the south side of the 43rd Street outfall ditch box culvert crossing SR 60 and the Cross-town Expressway, junction NA0300 in the model. This boundary condition was set at a constant elevation of 3.57 ft, NAVD based on staining of the endwall. The three other boundary conditions are at inter-connections between adjacent drainage systems. Two of these are located just north of SR 60 on the east side of the study area (junctions NA0965 and NA0980 in the model), and one is located along Hillsborough Avenue at the north end of the study area (junction NA0305 in the model). For all three locations the boundary conditions are set at constant elevations at which significant overland flows would occur leaving the drainage basin under study. The reasoning is to set tailwater conditions that would not likely be exceeded, thereby, generating conservative flows leaving the study area at these inter-connects. Links connected to these boundary junctions were set for positive flow only to prevent backflows into the model.

Table 4 – Boundary Conditions

Junction Name			Tailwater	
at Boundary			Elevation	Rationale for Setting
Condition	Location	Description of Boundary Condition	(ft, NAVD)	Tailwater Elevation
	McKay Bay, connected to Tampa	primary outfall for 43rd Street		
NA0300	Bay	outfall ditch	3.57	staining on endwall
	SR 60 storm sewer system, south			
	end of basin just east of primary	inter-connect with 29th Street		
NA0965	outfall NA0300	outfall, east side of study area	6.50	top of drainage structure
	Acline Drive south side ditch, just	inter-connect with 29th Street		
NA0980	north of NA0965	outfall, east side of study area	5.70	top of ditch bank
	Hillsborough Avenue storm sewer	inter-connect with larger storm		
	system, north end of basin, east	sewer that travels east then north		
NA0305	side	to Hillsborough River	49.40	overland flow elevation

3. EXISTING CONDITIONS

3.1. Model Verification

The model was verified by comparing the documented flooding from the July 8, 2011 storm event against model peak stages for the storm event that most closely matches that total rainfall depth. The recorded rainfalls for this event vary from 3.7 to 5.0 in the region with storm durations from 6 hours to 11 hours. The modeled storm that most closely matches the actual event at the study area is the 5-year, 4-hour storm event with 4.32 inches of rainfall. There are four locations where flooding was documented by photograph or video with the estimated flood elevation compared to the modeled peak stage in Table 5 below.

Table 5 – Flood Stages Comparison for Model Verification

Junction		Floor	d Stage	
Name	Location	(ft, NAVD)		Source
		5-Year,		
		4-Hour	Observed	
		HART Fac	ility	
				estimated 6-inch flood depth in
	Northwest corner of HART			service bays, 28.6 service bays
NA0620	facility	29.23	29.2	finished floor
				estimated 1.75-foot deep (half way
	Southeast corner of HART			up bus tires), 26.8 low pavement
NA0650	facility	28.32	28.55	elevation in parking area
	43rd 9	treet Out	fall Ditch	
	Upstream side of 10th Avenue			based on extent of flooding over
NA0220	crossing	27.44	27.0	roadway in video
	Upstream side of 43rd Street			estimated 6-inch depth sheet flow
NA0255	crossing	18.66	19.0	over 18.5 low top of ditch bank

3.2. Critical Duration Analysis

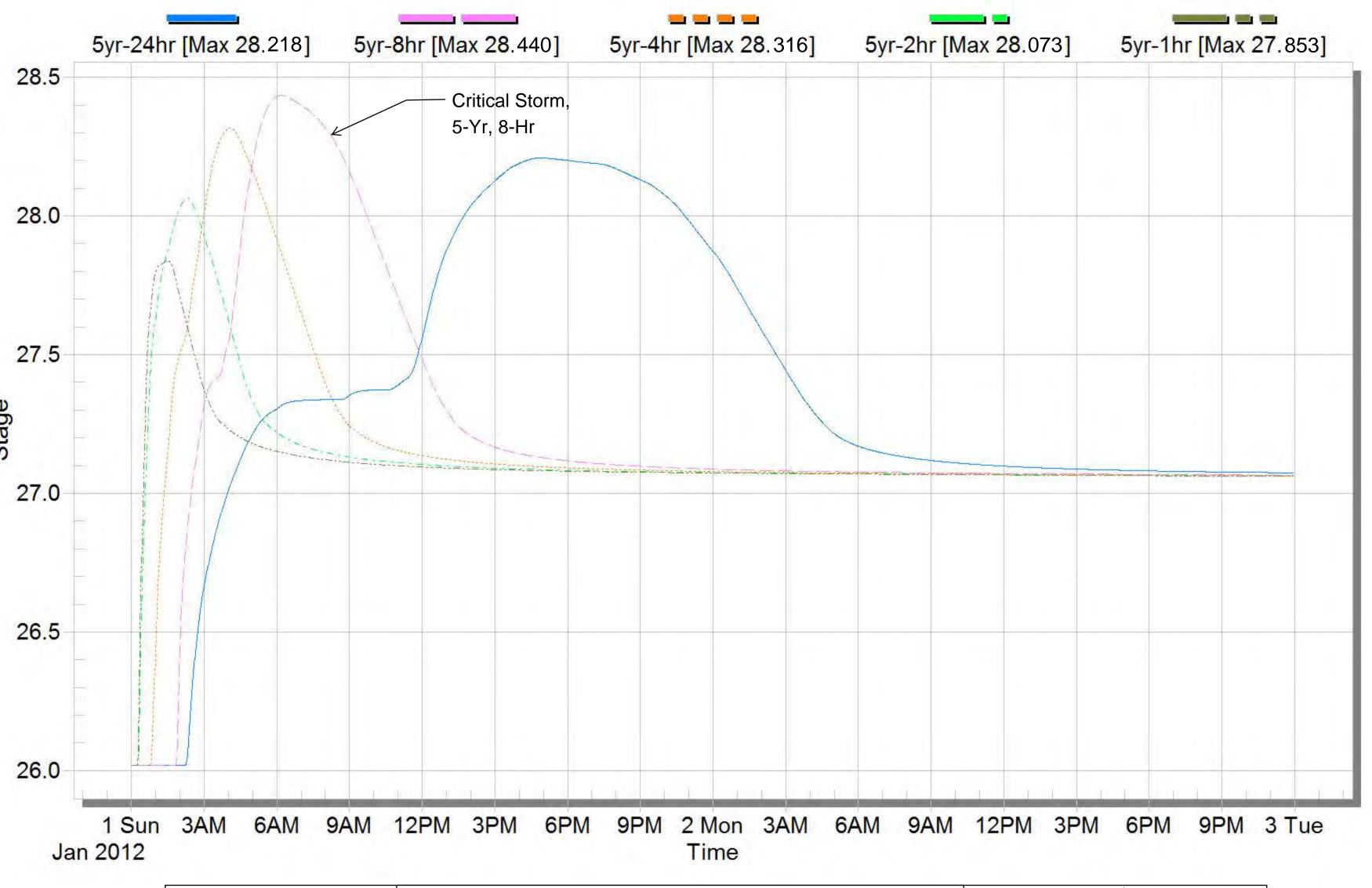
Pursuant to City requirements a critical duration analysis was performed for a series of 5-year storm events with varying durations and total rainfalls as listed in Table 6 below.

Table 6 – Critical Duration Analysis 5-Year Frequency Storms

Storm Duration	Rainfall Depth (inches)
1-hour	2.80
2-hour	3.50
4-hour	4.32
8-hour	5.36
24-hour	7.44

For this study, the critical storm is considered the duration that produces the most severe flooding at the focus area along the 43^{rd} Street outfall ditch between the HART facility and the 43^{rd} Street crossing. For the focus area, the critical storm is the 5-year, 8-hour storm. Figure 6 on the following page is a graph showing the time-stage relationships for each of the 5-year storms for the model junction at the HART facility. The 5-year, 8-hour peak stage is also the highest stage at all other junctions within the focus area.

Node - NA0650



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43rd Street Outfall Basin Study
Time-Stage Graph at HART Facility, Junction NA0650
(Determination of 5-Year Storm Critical Duration)



Figure 6

3.3. Existing Condition Model Results

Table 7 below is a summary of peak stage and flow conditions at selected nodes in the focus area of the study along the 43rd Street outfall ditch between the HART facility and the 43rd Street crossing. Critical elevations are included for determination of flooding severity.

Table 7 – Peak Stages at Select Nodes in Focus Area for Existing Conditions

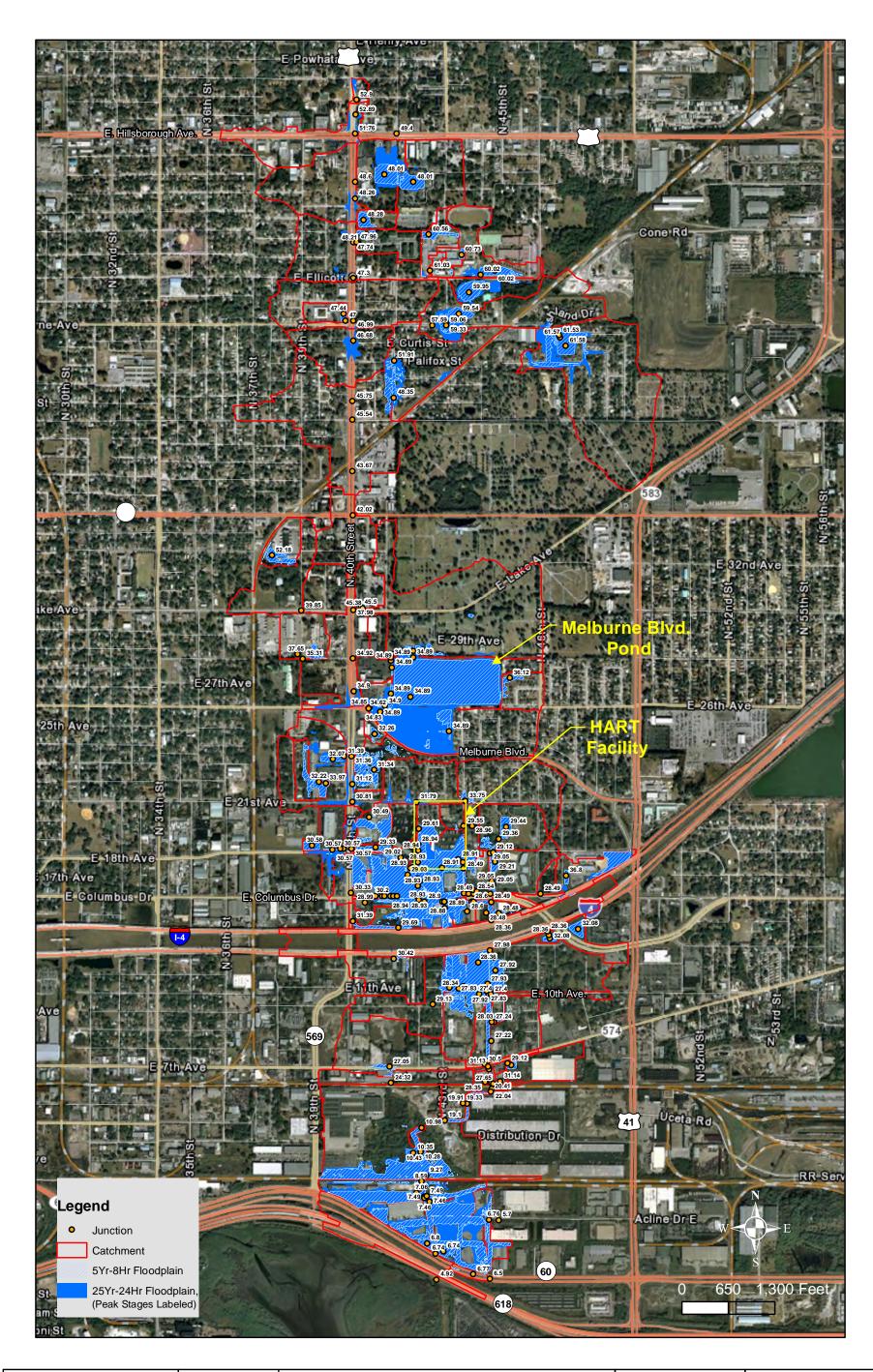
Junction Name	Location		Stage IAVD)	Critical Elevation
		5-Year,		
		8-Hour	24-Hour	
	HART F	acility		
NA0620	Northwest corner of HART facility	29.33	29.61	28.6 service bays finished floor
				26.9 low pavement elevation in
NA0655	Southeast corner of HART facility	28.32	28.79	parking area
	HART facility outfall ditch, north side			
NA0605	of 19th Avenue crossing	28.44	28.93	27.8 low edge of pavement
	43rd Street C	Outfall Di	tch	
NA0210	Upstream side of I-4 crossing	27.86	28.36	28.0 top of ditch bank
NA0215	Downstream side of I-4 crossing	27.61	27.98	28.0 top of ditch bank
NA1005	Upstream side of 10th Avenue crossing	27.52	27.83	26.0 low edge of pavement
NA0230	Upstream side of 7th Avenue crossing	25.93	27.22	30.1 top of ditch bank
NA0240	Upstream side of railroad crossings	25.02	26.35	29.1 low point of rail bed
NA0255	Upstream side of 43rd Street crossing	18.84	19.10	18.5 low top of ditch bank

Shading indicates flooding at junction.

Peak stages and flows for all of the modeled storms under existing conditions are included in Appendix A and attribute tables for junctions and links, respectively, in the GIS database.

3.4. Predicted Flooding under Existing Conditions

Floodplains for existing conditions have been delineated for the 5-year, 8-hour (critical storm) and the 25-year, 24-hour storm events based on peak stages generated from the model, see Figure 7 on the following page for floodplains shown on the aerial view. In the focus area, significant roadway flooding with overtopping occurs for both mapped storms due to ditch bank overtoppings along 19th Avenue and Columbus Drive. Roadway flooding and overtopping also occurs for both storms at the 43rd Street outfall ditch crossing with 10th Avenue.



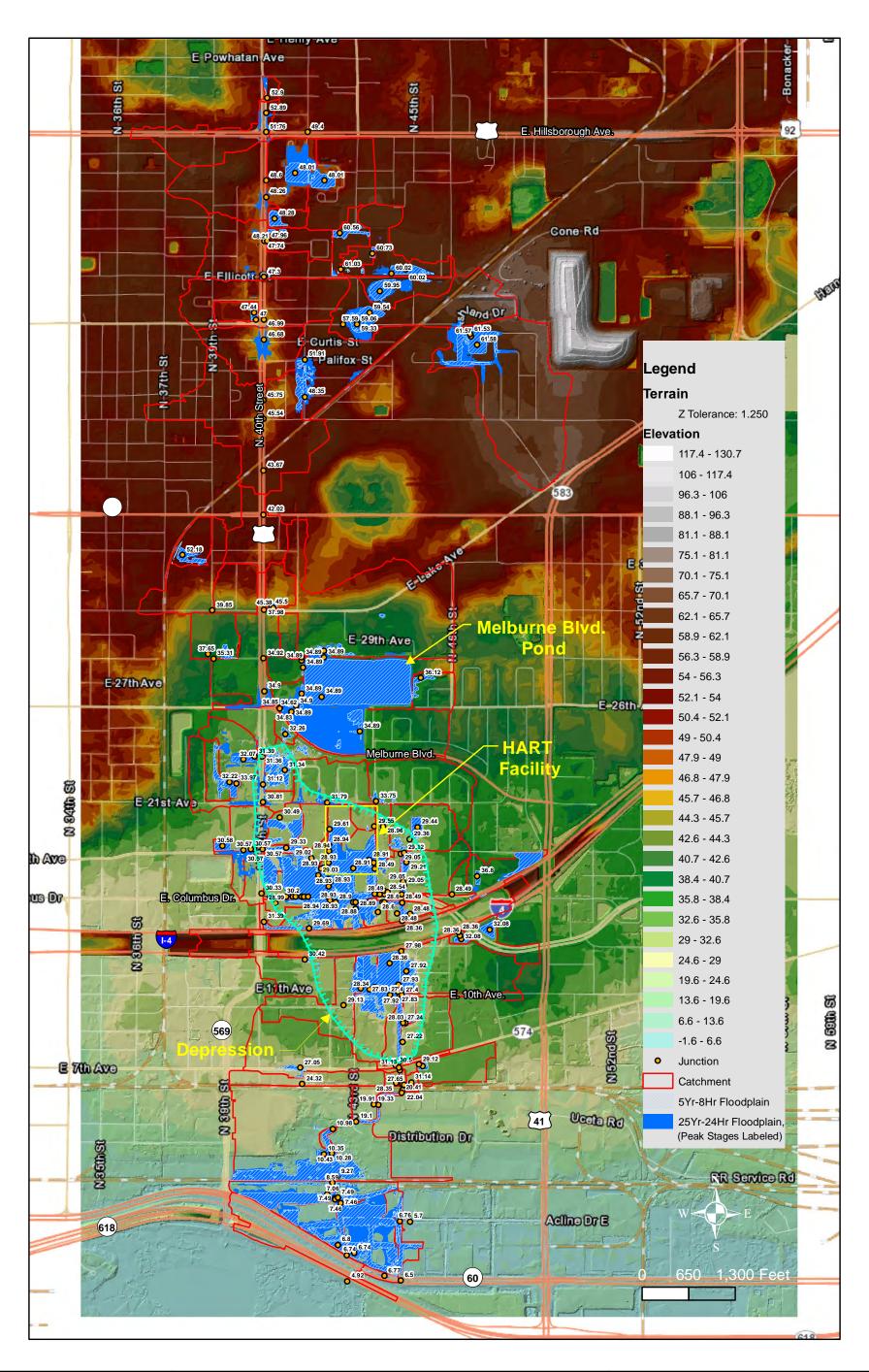




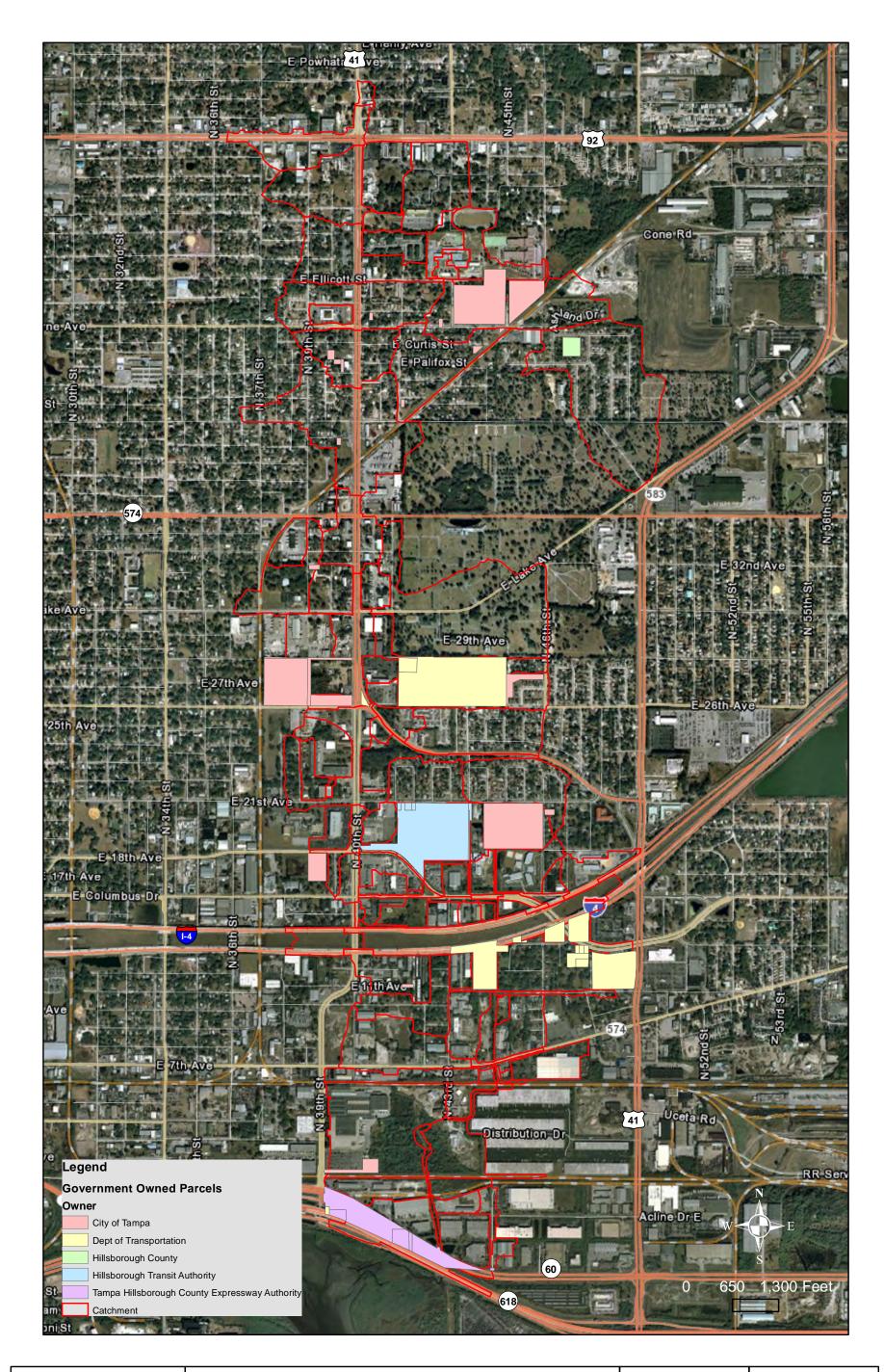
A large expanse of flooding over commercial sites, including the HART facility and others, occurs on both sides of I-4 in the vicinity of these roadway flooding areas. From review of the topography of this area, the cause of this flooding appears to be due to inadequate conveyance exiting a depression that is bisected by I-4. The 43rd Street outfall ditch crossings at 7th Avenue and adjacent railroad tracks appear to be the point of constriction. See Figure 8 on the following page for a view of the floodplains over terrain to see the floodplain extents within the depression and the point of conveyance constriction. This phenomenon is also documented by video during the July 8, 2011 event as high velocity flows were seen on the south side of 7th Avenue. A more localized flooding problem also appears at the south end of the focus area at the 43rd Street ditch crossing. At this location, flows overtop the ditch banks and across properties due to an undersized culvert along the ditch.

At the south end of the study area (outside of focus area), there also appears to be flooding for both the 5-year, 8-hour and 25-year, 24-hour storms of commercial areas and a City street (Acline Drive), and overtopping of railroad tracks crossing. Several large buildings may be at risk of flooding for the 25-year, 24-hour event.

Flooding also appears to the north of the focus area. For the 25-year, 24-hour storm, the Melburne Boulevard stormwater pond appears to overtop the banks and cause flooding of the entire Ace AA Used Auto Parts facility. Flooding also appears along several sections of a storm sewer system connecting two stormwater ponds to the 40th Street storm sewer including sections of Chelsea Street, 42nd Street, Osborne Avenue, and Ashland Drive. At the eastern pond on Ashland Drive, street flooding occurs for both mapped storms, and residential structures may be at risk for the 25-year storm. At the western pond on Osborne Avenue, both streets and residences appear to be at risk for both of the mapped storms. More street flooding appears to the south along 42nd Street for both mapped storms.











4. PROPOSED CONDITIONS

4.1. Proposed Improvement Alternatives

Three sets of preliminary alternatives have been developed and compared for improving flood protection in the problem areas identified in this study. These three preliminary alternatives are depicted in Figures 10, 11, and 12, and they are generally described below. Additionally, water surface profiles of the 43rd Street Outfall Ditch segment through the focus area are included for the three alternatives and existing conditions for the critical storm (5-year, 8-hour) in Appendix C.

Alternative 1: Conveyance Upgrades with South Pond

- Channel widening and reconstruction (varying widths up to **34 feet**) from HART Facility southward to 43rd Street crossing, distance of 3,970 linear feet.
- Installation of fabric filled concrete rip-rap, or other armoring, from I-4 crossing to 43rd Street crossing, distance of 2,740 linear feet.
- Acquisition of additional **4.3 acres** (affecting 16 properties) of right-of-way or easements for channel/culvert work.
- Secondary trunk line along Columbus Drive, (2) 4' x 10' box culverts for distance of 1,100 linear feet.
- Culvert crossing upgrades with multiple box culverts/pipes at **six** (6) roadway crossings and **two** (2) railroad crossings.
- Construction of **9.4 acre** stormwater pond for attenuation and treatment (South Pond).

Alternative 2: Conveyance Upgrades with North and South Ponds

- Channel widening and reconstruction (varying widths up to **15 feet**) from HART Facility southward to 43rd Street crossing, distance of 3,970 linear feet.
- Installation of fabric filled concrete rip-rap, or other armoring, from I-4 crossing to 43rd Street crossing, distance of 2,740 linear feet.
- Acquisition of additional **3.3 acres** (affecting 14 properties) of right-of-way or easements for channel/culvert work.
- Secondary trunk line along Columbus Drive, (2) **48-inch pipes** for distance of 1,100 linear feet.
- Culvert crossing upgrades with multiple box culverts/pipes at **six** (6) roadway crossings and **two** (2) railroad crossings.
- Construction of **9.4 acre** stormwater pond for attenuation and treatment (South Pond).
- Construction of **7.1 acres** stormwater pond for flood storage and treatment (North Pond).

Alternative 3: Diversion System with North and South Ponds

- Diversion system along 40th Street with **4' x 6' box culvert**, distance of 3,900 linear feet.
- Acquisition of additional **0.4 acres** (affecting 4 properties) of right-of-way or easements for culvert work.
- Culvert crossing upgrades with single box culverts/pipes at **two** (2) roadway crossings and **one** (1) railroad crossing.
- Construction of **9.4 acre** stormwater pond for attenuation and treatment (South Pond).
- Construction of **7.1 acres** stormwater pond for flood storage and treatment (North Pond).

Note that additional conveyance (pipe) and inlet upgrades within the HART Facility may be beneficial to improve protection of portions of the property from localized flooding.

4.2. Proposed Stormwater Ponds

Following are characteristics of the preliminary designs of these ponds with the primary design functions in the headings (elevations in ft, NAVD88):

North Pond (7.1 Ac) – flood storage

- Off-line Wet Detention
- Property acreage = 7.1 acres (South Side of Columbus Drive)
- Berm Elevation = 27.5
- Control Elevation = 25.0
- Bottom Elevation = 16.5 (max. depth of 8.5 feet)
- Permanent Pool Volume = 39.9 ac-ft

It should also be mentioned that for Alternative 3, the North Pond may be designed with a bleed-down device at a lower control elevation (24.0 or lower) with a connection to the proposed diversion box culvert on 40th Street. Adding one or more feet vertically of additional storage in the pond may further improve flood protection and allow reducing the size of the 4' x 6' diversion box culvert. Additionally, a flap gate could be considered on the box culvert connection to the Columbus Drive ditch to direct and capture large storm flows into the pond. A small bleed-down pipe could be installed in parallel to allow slow and controlled recovery of the pond back into the ditch.

South Pond – attenuation and treatment

- Off-line/on-line Wet Detention
- Property acreage = 9.4 acres
- Berm Elevation = 20.0
- Shelf Elevation = 9.0
- Control Elevation = 6.0
- Bottom Elevation = -1.0 (max. depth of 7.0 feet)
- Permanent Pool Volume = 20.8 ac-ft

The South Pond could be designed to maximize treatment flow path with the addition of a small bleed-down device toward the southeast corner of the pond with connection to the adjacent outfall ditch.

All three alternatives provide significant water quality benefits to the receiving water of Tampa Bay through detention of stormwater in the permanent pools in the combinations of proposed North and South Ponds. The 43rd Street outfall is located in the Palm River Basin as delineated by FDEP (WBID 1536E). This basin is listed and verified as impaired for Dissolved Oxygen and Nutrients.

Please note that site-specific geotechnical investigations will be necessary to establish seasonal high/normal groundwater levels and possibly base seepage flows across the potential pond sites for use in final design. Contamination investigations over all potential pond sites are strongly encouraged early in the design process due to the historical industrial uses of the area. Additionally, potential for hazardous materials (such as asbestos or others) to exist in the existing buildings to be demolished for pond sites should be considered in evaluating costs and use of potential pond sites.





February, 2013





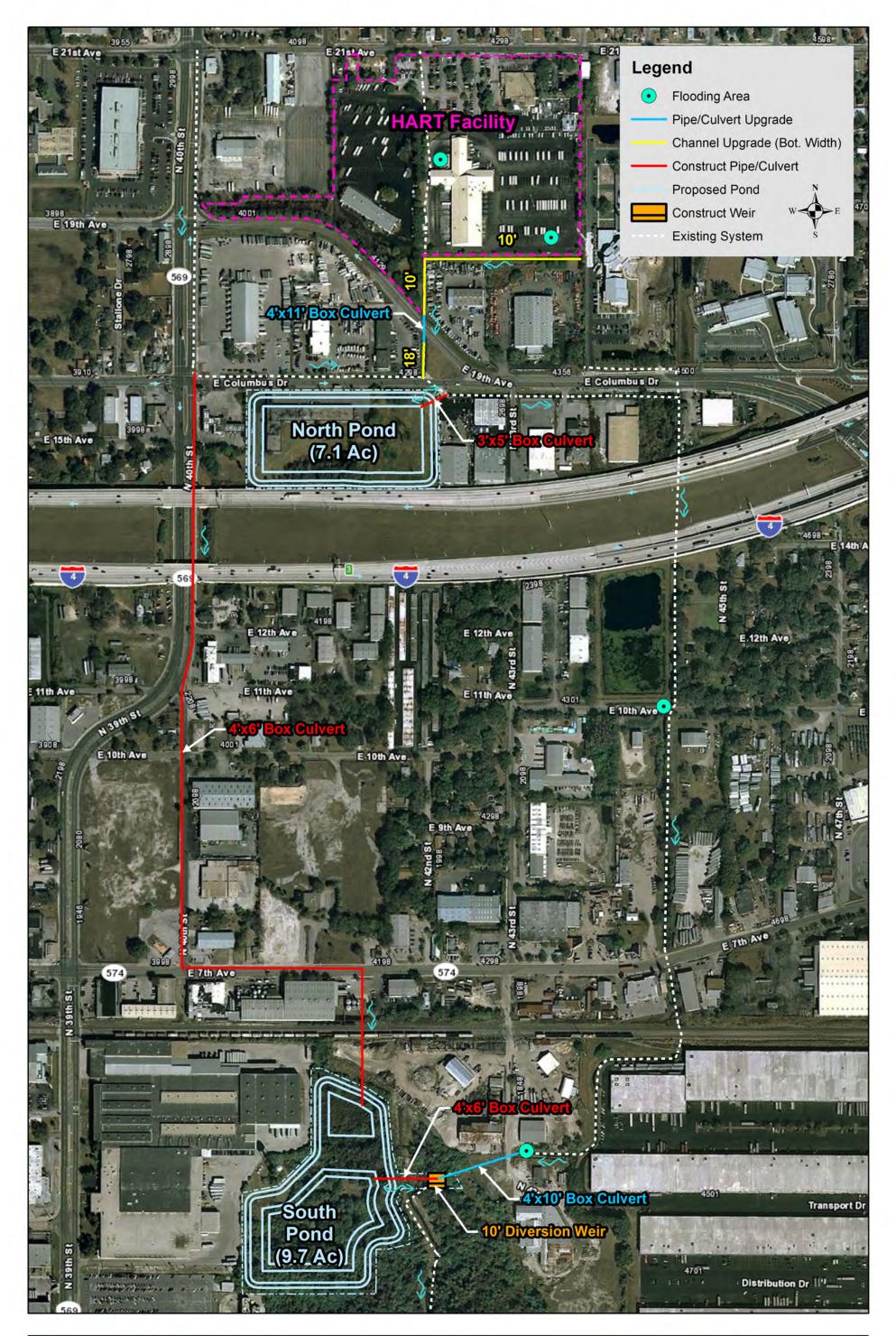


43rd Street Outfall Basin Study

Alternative 2 - Conveyance Upgrades with North & South Ponds









4.3. Proposed Conditions Model Results

The three alternatives have been determined in order to provide flood protection at the HART site, all roadway crossings, and private properties along the 43rd Street outfall ditch alignment within the focus area of the study between the HART site and the 43rd Street crossing. No adverse effects from proposed improvements are expected upstream of the focus area. Additionally, the south pond is included in the alternatives for the purpose of fully attenuating the increase of storm flows due to proposed channel and culvert upgrades so that downstream portions of the system and private properties see no adverse effects from the project.

The expected level of service for flood protection is the critical duration of the 5-year recurrence interval, identified as the 8-hour duration storm. Peak stages of the alternatives presented are below critical low pavement areas or within the top of ditch banks as further detailed in the table included below.

Table 8 – Peak Stages at Select Nodes in Focus Area for Proposed Conditions

Junction			ak Stage	(ft, NAVD		
Name	Location		5-Year, 8-Hour		Critical Elevation	
		Existing	Alt. 1	Alt. 2	Alt. 3	
		HART F	cility			
						26.9 low pavement elevation in
NA0655	Southeast corner of HART facility	28.32	26.85	26.88	26.89	parking area
	HART facility outfall ditch, north side					
NA0605	of 19th Avenue crossing	28.44	26.82	26.84	26.84	27.8 low edge of pavement
	43	rd Street O	utfall Dit	ch		
NA0210	Upstream side of I-4 crossing	27.86	25.92	26.03	26.21	28.0 top of ditch bank
NA0215	Downstream side of I-4 crossing	27.61	24.81	25.70	26.12	28.0 top of ditch bank
NA1005	Upstream side of 10th Avenue crossing	27.52	24.38	25.39	26.05	26.0 low edge of pavement
NA0230	Upstream side of 7th Avenue crossing	25.93	21.78	23.13	25.61	30.1 top of ditch bank
NA0240	Upstream side of railroad crossings	25.02	19.39	20.86	24.56	29.1 low point of rail bed
NA0255	Upstream side of 43rd Street crossing	18.84	16.50	16.79	17.12	18.5 low top of ditch bank

A critical aspect of this project will be to ensure that no adverse impacts occur downstream of the proposed improvements due to improving conveyance of the existing system. The South Pond is included in all alternatives and sized in order to provide attenuation so there is no increase in peak flows, as shown in the following table. The 25-year, 24-hour storm event was analyzed to show compliance with the attenuation requirement in SWFWMD permitting rules.

Table 9 – Peak Flows Comparison for Proposed Conditions

	Link Name		Peak Flow (CFS)										
		Location		5-Year,	8-Hour		25-Year, 24-Hour						
			Existing	Alt. 1	Alt. 2	Alt. 3	Existing	Alt. 1	Alt. 2	Alt. 3			
		43rd St Outfall											
	RA0280	Ditch, South of	399	353	317	319	491	484	448	460			
		Railroad											

4.4. Discussion and Comparison of Alternatives

Three feasible alternatives have been developed to provide comparable and reasonable levels of flood protection within the focus area of the study. Advantages and challenges are found with all three alternatives.

More specifically, Alternatives 1 and 2 focus on upsizing and improving conveyance of the existing 43rd Street outfall ditch. Increases of slope and conveyance also lead to increases of velocities in the channel to the extent that channel lining is recommended from the I-4 crossing southward to the 43rd Street crossing for a distance of 2,740 linear feet. For cost estimating, fabric-filled concrete rip-rap has been identified, however, other types of armoring could be explored such as articulating block or ditch pavement. Additionally, for Alternative 1 and to a lesser extent Alternative 2, right-of-way acquisitions or easements will be necessary for up to 4.3 acres involving 16 property owners along the banks of the existing channel segments. These negotiations could prove challenging to manage within a scheduled timeframe and cost, especially if grant funding and obligations are involved.

The main component of Alternative 3 is a proposed new diversion 4' x 6' box culvert along 40th Street, a portion of which is FDOT State Road 569. For Alternative 3, early coordination with FDOT will be important to gain FDOT's commitment to this conceptual plan.

Construction of large box culvert under active railroad tracks (or bore and jack equivalent pipes) owned by CSX railroad is a component of all three alternatives that is expected to be complex in design, approval, and construction. Alternative 3 provides an advantage concerning railroad issues in that the box culvert size is 4' x 6', as opposed to up to triple 4' x 10' for Alternative 1.

Utility relocations and conflict resolutions are expected to be difficult but not prohibitive for all three alternatives. See Appendix F for more detailed information about existing utilities. Both Alternative 2 and 3 involve negotiations with up to three separate property owners for construction of the North and South Ponds. These negotiations may lead to uncertainty in the schedule and project costs. An alternative site for the North Pond is also feasible on the north side of Columbus Drive, should the preferred location as shown on exhibits prove infeasible.

4.5. Estimated Alternative Costs

Detailed cost estimates are included for all three alternatives in Appendix C. Most construction unit prices are based on FDOT average annual construction contract values tabulated for the year 2011. Property cost estimates are further detailed in Appendix E, and are based on market or assessed property values displayed on the Hillsborough County Property Appraiser's website. For some of the properties, recent sales values are also available for comparison. Alternative 3 is estimated as the least cost alternative at \$8,963,000 including design, property acquisition, and construction costs.

Table 10 – Summary of Estimated Project Costs for Alternatives

Alternative	Estima	Estimated Project Cost			
1: Conveyance Upgrades with South Pond	\$	9,110,000			
2: Conveyance Upgrades with North and South Ponds	\$	9,149,000			
3: Diversion System with North and South Ponds	\$	8,963,000			

4.6. Recommendation and Phasing

The scope of the flooding problem is large and a major commitment to improve the situation will be necessary. We understand the City is planning to partner for funding and divide the improvements into multiple phases to reach the desired level of flood protection in the study area.

Though estimated costs are close among the alternatives, the recommended course of action to provide necessary flood protection for the study area is **Alternative 3 with a diversion system along 40th Street and North and South Ponds**. The main reasons for this choice of recommendation are; 1) involvement with many fewer property owners (three as opposed to up to 19) that are critical to the project success, (2) avoidance of major disruptions to the 43rd Street outfall ditch, (3) and further distribution or splitting of stormwater flows instead of promotion of higher flow rates.

Following is a schedule of possible phasing for the recommendations. Each phase may be subdivided into property acquisitions and design/construction sub-phases.

Table 11 – Phasing of Recommendations

	Esti	mated Property	Estimated Design and			Total Estimated		
Phases		Costs	Co	onstruction Costs		Phase Cost		
1: Construct North Pond and connection to HART	\$	1,500,000	\$	1,266,000	\$	2,766,000		
2: Construct South Pond, diversion weir, and								
box culvert across 43rd Street	\$	400,000	\$	1,754,000	\$	2,154,000		
3: Construct diversion system along 40th Street								
to South Pond	\$	57,000	\$	3,986,000	\$	4,043,000		

Total = \$ 8,963,000

Appendix A

Existing Conditions XP-SWMM Model Input and Results

(under separate cover)

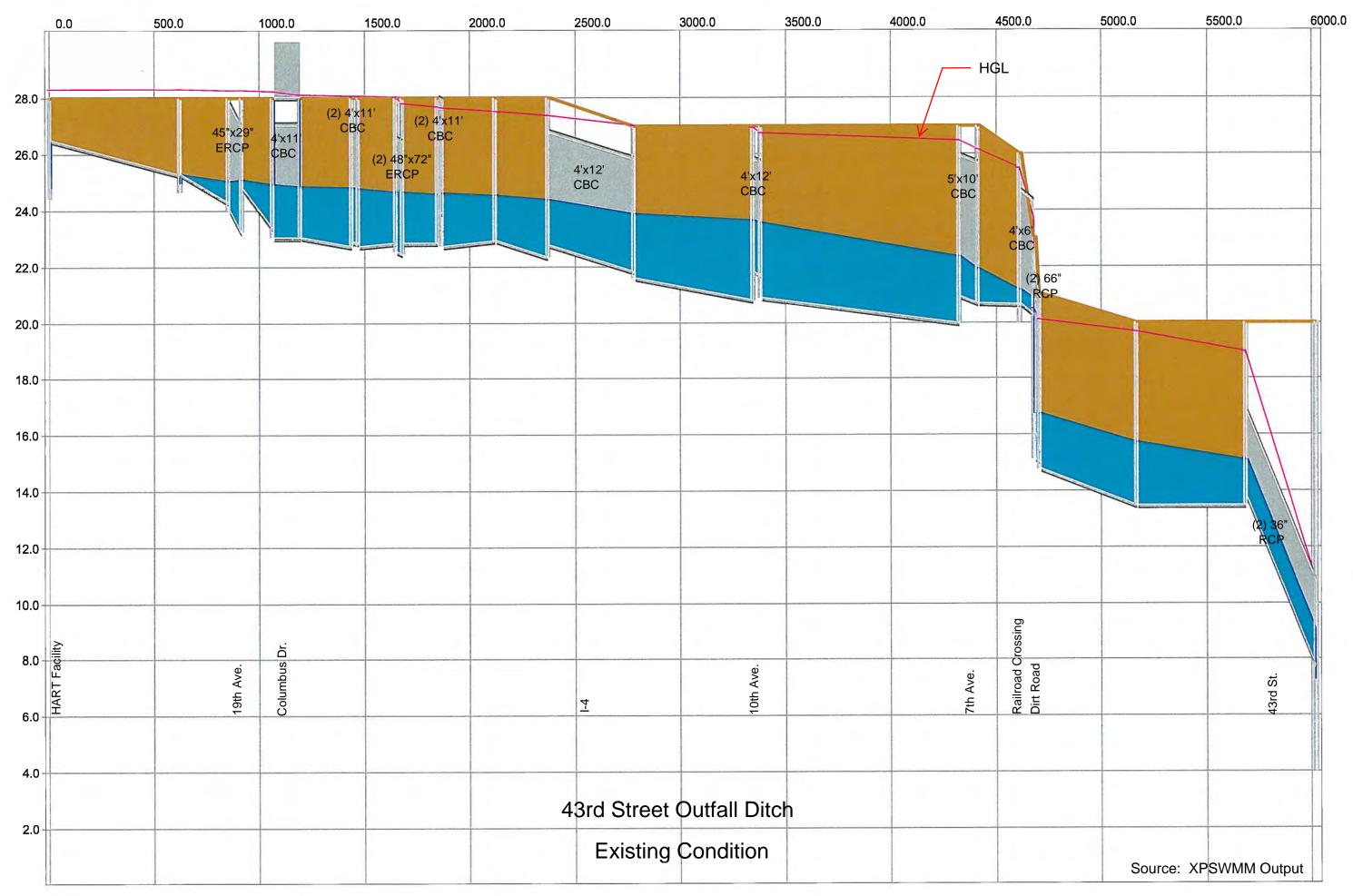
Appendix B

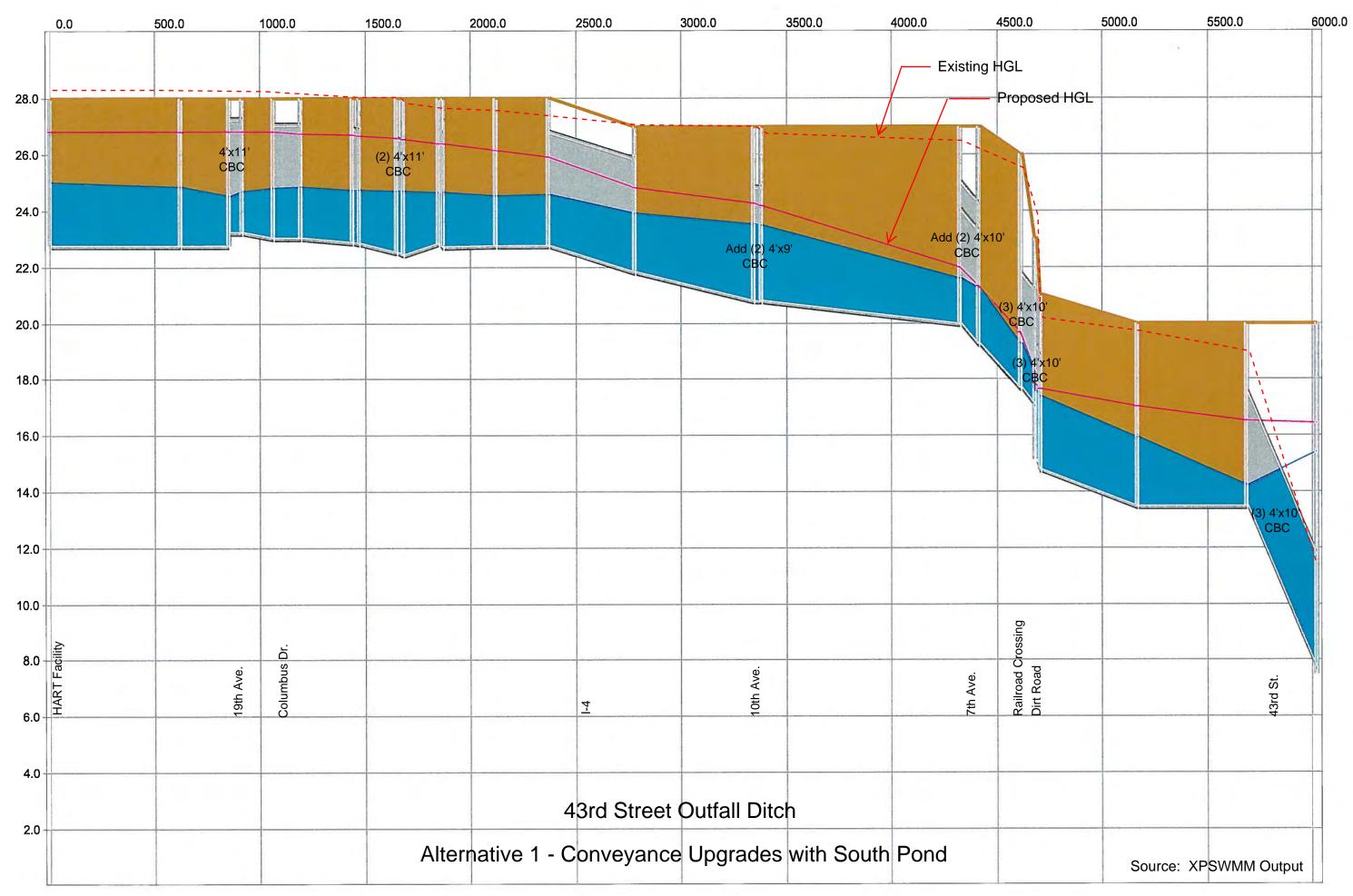
Proposed Conditions XP-SWMM Model Input and Results

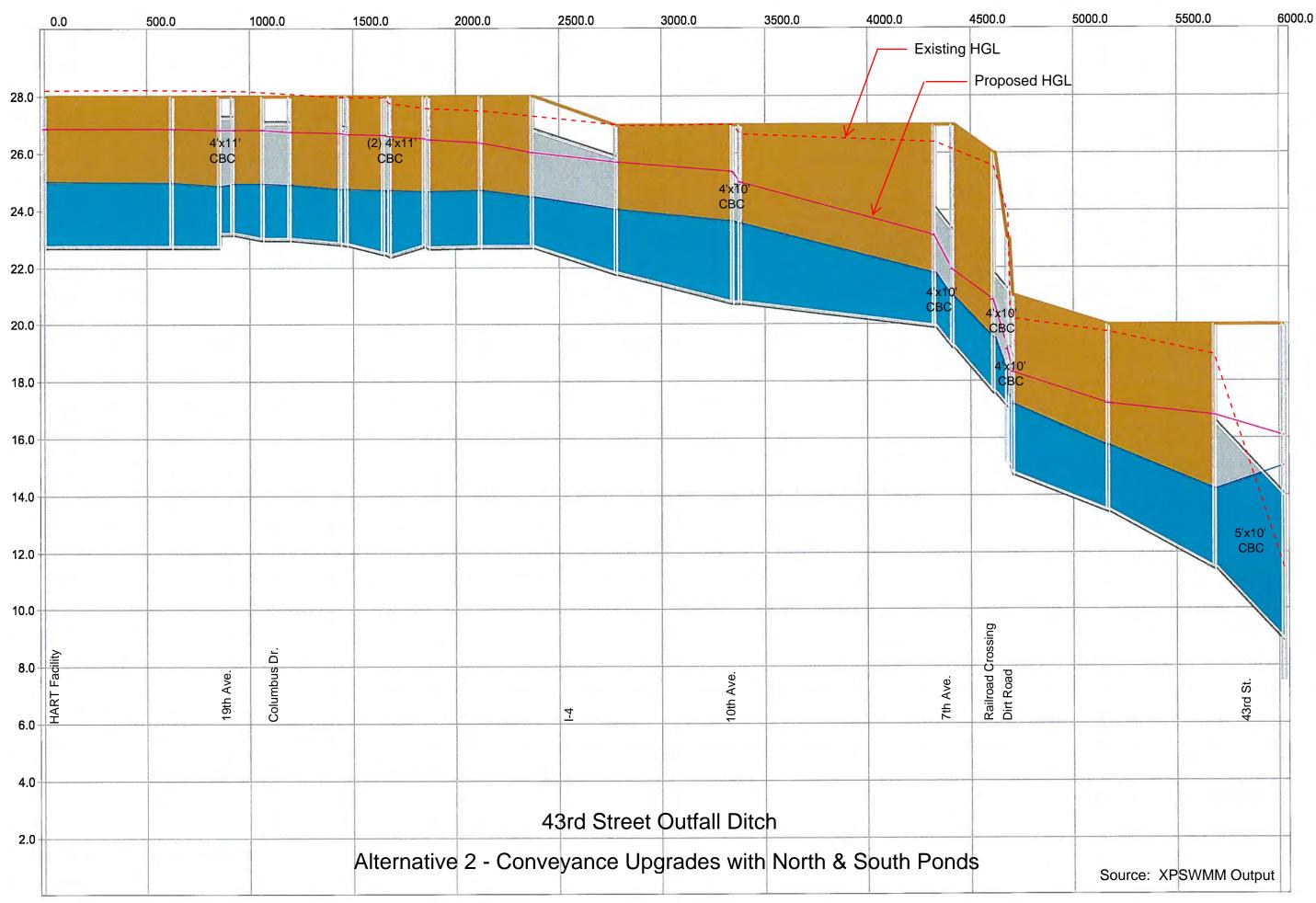
(under separate cover, includes Alternative 4)

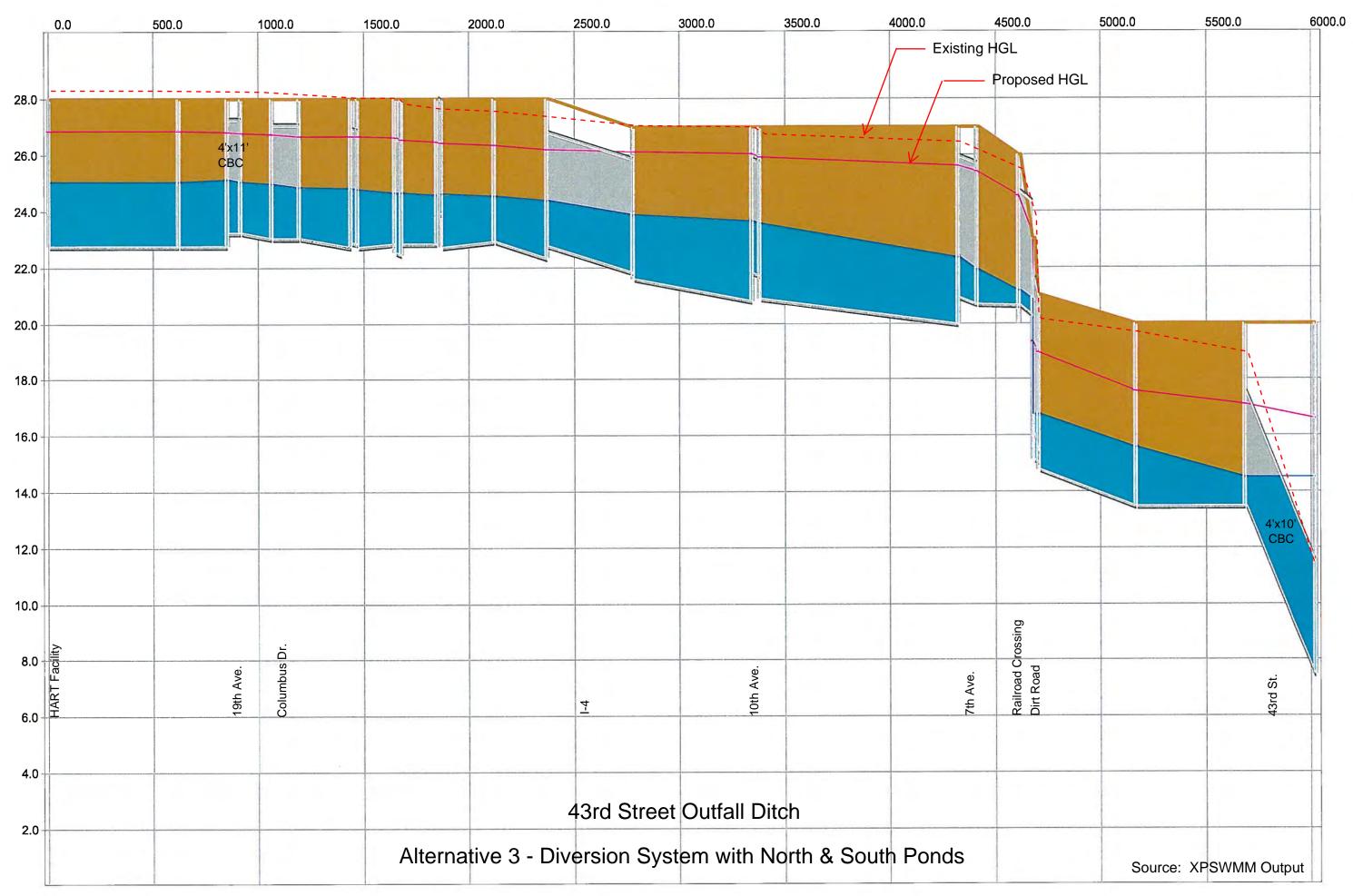
Appendix C

43rd Street Outfall Ditch Profiles for Existing and Proposed Conditions









Appendix D

Preliminary Project Cost Estimates

Engineer's Estimate of Project Costs for Improvements to 43rd Street Outfall Ditch Basin

Alternative	1	Convovence	Ilnanodo	a with	Courth	Dand
Allernative	-	Convevance	Ungrade	s wiin	Somm	rona

<u>Item Number</u>	<u>Description</u>	<u>Unit Cost</u>	<u>Units</u>	Quantity	Total Cost				
1	Clearing and Grubbing	\$7,500.00	AC	20.6	\$154,500				
2	Erosion Control	\$10,000	LS	1	\$10,000				
3	Structure with 10-foot Diversion Weir	\$10,000.00	EA	1	\$10,000				
4	Manhole Riser (for access to box culvert)	\$3,000.00	EA	2	\$6,000				
5	Concrete for Endwall	\$670.00	CY	137.0	\$91,790				
6	Building Demolition (1 story with small machinery)	\$4.50	SF	13,800	\$62,100				
7	Box Culvert, 4' x 6'	\$526.00	LF	230	\$120,980				
8	Box Culvert, 4' x 9'	\$684.00	LF	68	\$46,512				
9	Box Culvert, 4' x 10'	\$736.00	LF	1,932	\$1,421,952				
10	Box Culvert, 4' x 11'	x 11' \$789.00	LF	129	\$101,781				
11	Box Culvert, 5' x 10'	\$789.00	LF	180	\$142,020				
12	Channel Excavation	\$6.80	CY	39,511	\$268,675				
13	Channel Lining with Fabric Formed Riprap	\$60.00	SY	21,174	\$1,270,467				
14	Pond Excavation	\$4.00	CY	111,830	\$447,320				
15	Pond Berm Embankment	\$5.00	CY	9,056	\$45,280				
16	Performance Turf (Sod)	\$1.70	SY	41,500	\$70,550				
17	Chain-link Fence (6')	\$13.00	LF	8,770	\$114,010				
18	Concrete Sidewalk (Driveway), 6-inch Thick	\$40.00	SY	472	\$18,880				
19	Concrete Sidewalk, 4-inch Thick	\$29.00	SY	392	\$11,368				
20	Roadway Open Cut Restoration	\$57.00	SY	858	\$48,906				
21	Roadway Lane Restoration	\$43.00	SY	2,583	\$111,069				
	Maintenance of Traffic (12%)				\$548,899				
	Mobilization & Demobilization (10%)				\$512,306				
	Contingency Cost (20%)				\$1,127,073				
	Construction Cost Sub-total				\$6,762,437				
	Construction Engineering Inspection and Testing (5%)				\$338,122				
	Engineering Design and Permitting (7%)				\$473,371				
	Railroad Compliance and Permitting	\$100,000.00	LS	1	\$100,000				
	Property Acquisition for Conveyance Upgrades and South Pond (see separate tabulation)								
	Estimated Project Costs, Alternative 1				\$9,110,000				

Engineer's Estimate of Project Costs for Improvements to 43rd Street Outfall Ditch Basin

Alternative	2	Commonan	TIm area d	laa wiith	Carreth an	d Month	Danda
Alternative	4 -	Convevance	UDYFAC	ies with	Soum an	a North	Ponas

<u>Item Number</u>	<u>Description</u>	<u>Unit Cost</u>	<u>Units</u>	Quantity	Total Cost					
1	Clearing and Grubbing	\$7,500.00	AC	25.7	\$192,450					
2	Structure with 12-foot Diversion Weir	\$10,000.00	EA	1	\$10,000					
3	Modify Drainage Structure (connect pipe)	\$1,500.00	EA	1	\$1,500					
4	Manhole Riser (for access to box culvert)	\$3,000.00	EA	2	\$6,000					
5	Concrete for Endwall	\$670.00	CY	118.0	\$79,060					
6	Building Demolition (with wrecking ball)	\$2.00	SF	58,800	\$117,600					
7	Building Demolition (with small machinery)	\$4.50	SF	13,800	\$62,100					
8	Reinforced Concrete Pipe (RCP), 48-inch	\$130.00	LF	2,350	\$305,500					
9	Box Culvert, 3' x 4'	\$400.00	LF	90	\$36,000					
10	Box Culvert, 4' x 6'	\$526.00	LF	230	\$120,980					
11	Box Culvert, 4' x 10'	\$736.00	LF	215	\$158,240					
12	Box Culvert, 4' x 11'	\$789.00	LF	129	\$101,781					
13	Box Culvert, 5' x 10'	\$789.00	LF	335	\$264,315					
14	Channel Excavation	\$6.80	CY	15,465	\$105,162					
15	Channel Lining with Fabric Formed Riprap	\$60.00	SY	15,162	\$909,733					
16	Pond Excavation	\$4.00	CY	212,194	\$848,776					
17	Pond Berm Embankment	\$5.00	CY	9,132	\$45,660					
18	Performance Turf (Sod)	\$1.60	SY	61,274	\$98,038					
19	Chain-link Fence (6')	\$13.00	LF	11,120	\$144,560					
20	Concrete Sidewalk (Driveway), 6-inch Thick	\$38.00	SY	472	\$17,936					
21	Concrete Sidewalk, 4-inch Thick	\$29.00	SY	392	\$11,368					
22	Roadway Open Cut Restoration	\$57.00	SY	755	\$43,035					
23	Roadway Lane Restoration	\$43.00	SY	2,000	\$86,000					
	Maintenance of Traffic (12%)				\$451,895					
	Mobilization & Demobilization (10%)				\$421,769					
	Contingency Cost (20%)				\$927,892					
	Construction Cost Sub-total				\$5,567,351					
	Construction Engineering Inspection and Testing (5%)				\$278,368					
	Engineering Design and Permitting (7%)				\$389,715					
	Railroad Compliance and Permitting	\$75,000.00	LS	1	\$75,000					
	Property Aquisition for Conveyance Upgrades and South/North Ponds (see separate tabulation)									
	Estimated Project Costs, Alternative 2				\$9,149,000					

Engineer's Estimate of Project Costs for Improvements to 43rd Street Outfall Ditch Basin

Alternative	3 -	Diversion	Syctom	with N	Jorth ar	d South	Donde
Allernative	. 7 -	Diversion	System	WHILL	ik minor	10 2011III	Ponds

<u>Item Number</u>	<u>Description</u>	<u>Unit Cost</u>	<u>Units</u>	Quantity	Total Cost			
1	Clearing and Grubbing	\$7,500.00	AC	21.5	\$161,400			
2	Structure with 10-foot Diversion Weir	\$10,000.00	EA	1	\$10,000			
3	Modify Drainage Structure (connect pipe)	\$1,500.00	EA	2	\$3,000			
4	Manhole Riser (for access to box culvert)	\$3,000.00	EA	12	\$36,000			
5	Concrete for Endwall	\$670.00	CY	34.8	\$23,329			
6	Building Demolition (with wrecking ball)	\$2.00	SF	58,800	\$117,600			
7	Box Culvert, 3' x 5'	\$421.00	LF	90	\$37,890			
8	Box Culvert, 4' x 6'	\$526.00	LF	3,910	\$2,056,660			
9	Box Culvert, 4' x 10'	\$736.00	LF	335	\$246,560			
10	Box Culvert, 4' x 11'	\$789.00	LF	67	\$52,863			
11	Channel Excavation	\$6.80	CY	1,867	\$12,696			
12	Pond Excavation	\$4.00	CY	212,194	\$848,776			
13	Pond Berm Embankment	\$5.00	CY	9,132	\$45,660			
14	Performance Turf (Sod)	\$1.60	SY	48,601	\$77,762			
15	Chain-link Fence (6')	\$13.00	LF	5,690	\$73,970			
16	Concrete Sidewalk (Driveway), 6-inch Thick	\$38.00	SY	981	\$37,278			
17	Concrete Sidewalk, 4-inch Thick	\$29.00	SY	1,190	\$34,510			
18	Roadway Open Cut Restoration	\$57.00	SY	375	\$21,375			
19	Roadway Lane Restoration	\$43.00	SY	6,714	\$288,702			
	Maintenance of Traffic (12%)				\$502,324			
	Mobilization & Demobilization (10%)				\$468,835			
	Contingency Cost (20%)				\$1,031,438			
	Construction Cost Sub-total				\$6,188,628			
	Construction Engineering Inspection and Testing (5%)				\$309,431			
	Engineering Design and Permitting (7%)				\$433,204			
	Railroad Compliance and Permitting	\$75,000.00	LS	1	\$75,000			
	Property Costs for Minor Conv. Upgrades and South/North Ponds (see separate tabulation)							
	Estimated Project Costs, Alternative 3				\$8,963,000			

Appendix E

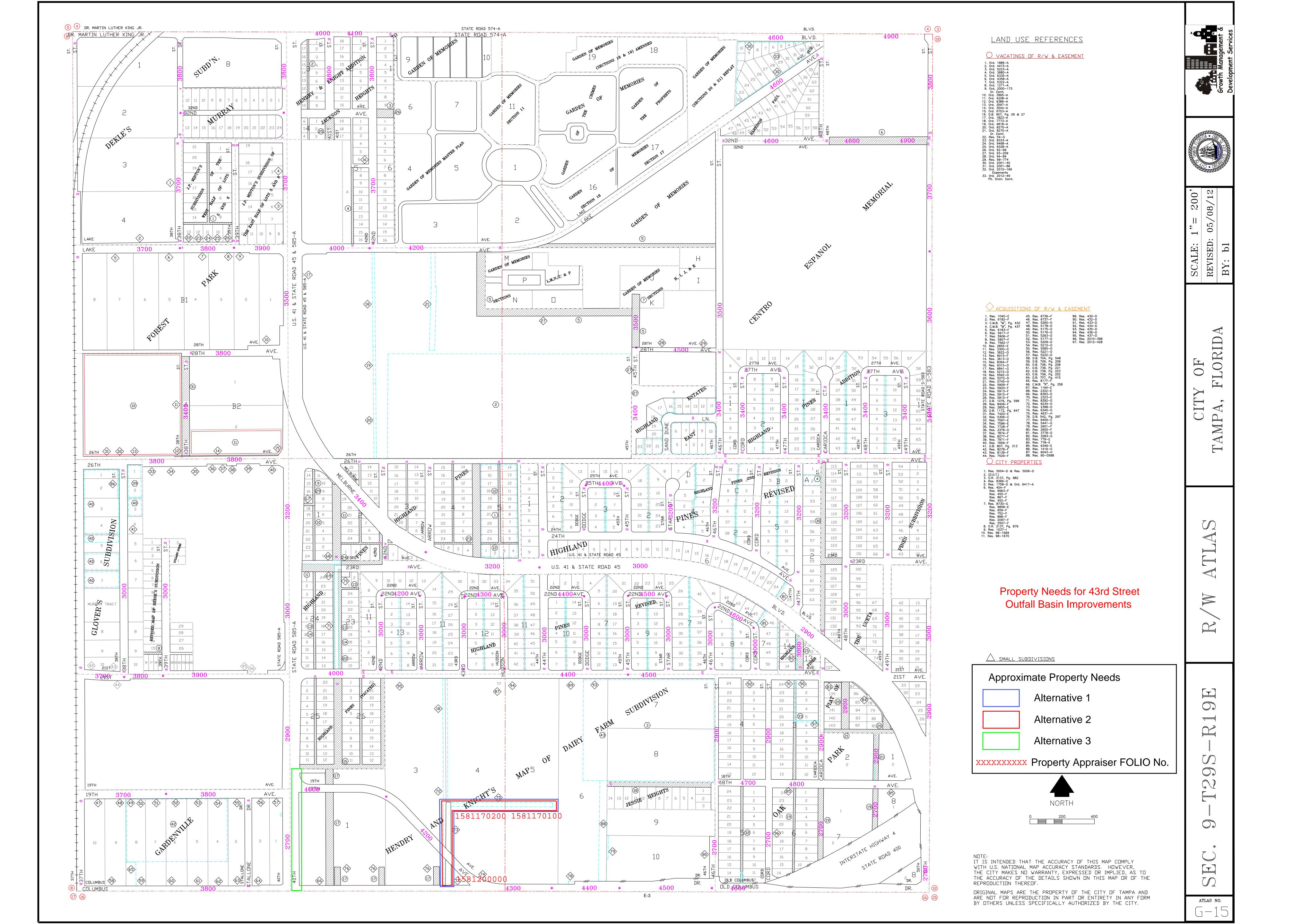
Property Needs Back-up Information

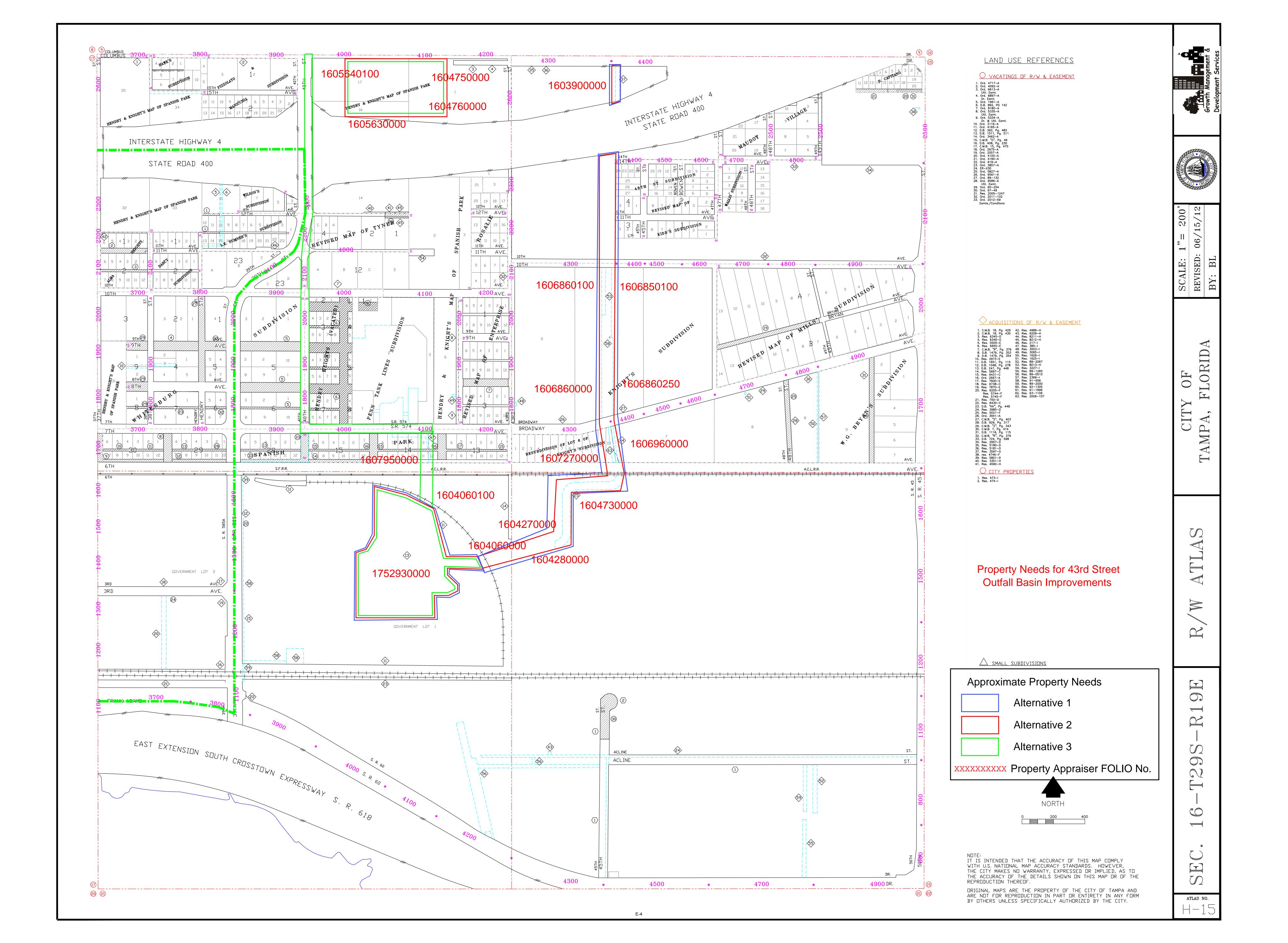
FOLIO	Owner	Size (acres) Assessed or Market Value		Buildings	Last Sale	Last Sale Date	Notes			
North Pond (South s	ide of Columbus Drive)									
4004	700000 D	0.40	\$000.040	Taxa-	#050.000	0/47/0000	T			
	760000 Bravura Investment Corp.	3.13			\$250,000	9/17/2003				
	750000 Ricknata, LLC	0.58	. ,		\$500,000	12/30/2010				
	630000 Ricknata, LLC	2.21	\$144,597				included above			
1605	640100 Ricknata, LLC	2.48	\$1,047,600	Yes			included above			
		8.40								
1581	050000 Old Dominion Freight Line, Inc. Estimated Property Purchase Price =	5.63		Yes	\$590,000		For Sale?			
	Estimated Property Purchase Price = Estimated Administrative/Legal Costs = Estimated Total Property Costs =			purchase price (from			options			
South Pond										
1752	930000 Southgate Tampa Industrial Ltd. Partnership	38.6 (using 9.7)	\$1,060,103	Yes (not affected)	N/A		land only (\$27,500 per acre)			
Estimated Property Purchase Price = \$266,750 based on market land value prorated for 9.4 acres out of 38.6 acres total Estimated Administrative/Legal Costs = \$133,375 50% of estimated purchase price (from sample of FDOT estimates) Estimated Total Property Costs = \$400,000										

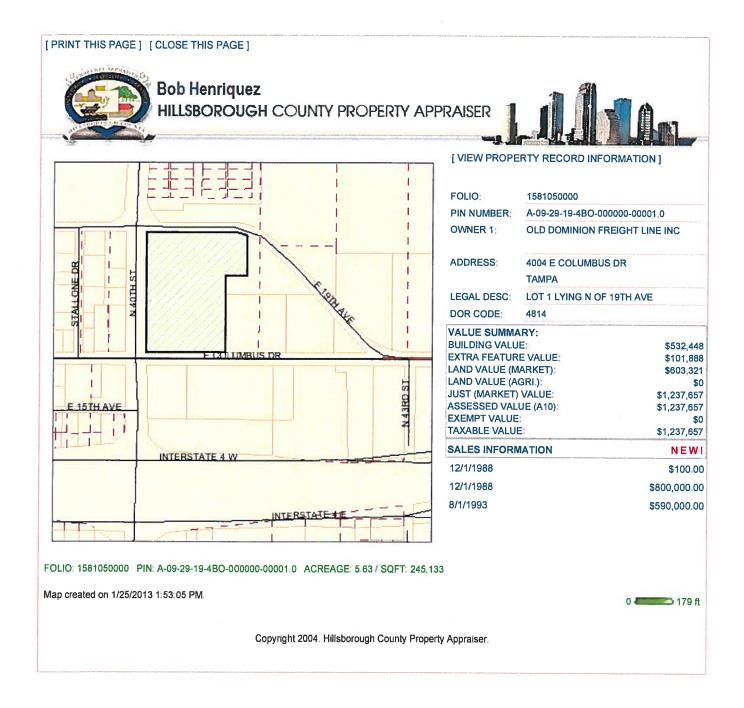
FOLIO	Owner	Size (acres)	Assessed	Buildings	Last Sale	Last Sale	Property Value	Rate	Alternativ	e 1 Impacts	Alternative	2 Impacts	Alternative 3	Impacts
			or Market Value			Date			Size (acres)	Cost	Size (acres)	Cost	Size (acres)	Cost
onveyance Upgrades														
	T property will not require property costs													
	Messina Holdings, Inc.	4.07		Yes (not affected)			\$105,000.00	per acre	0.08	\$8,400	0.04	\$4,200	0.04	\$4,200.
	Messina Properties, Inc.	2.39	\$251,965	Yes (not affected)	N/A		\$105,000.00	per acre	0	\$0	0	\$0	0	\$0.
1581200000	Bowman Transportation, Inc.	0.32	\$45,000	No	N/A		total purchase	total	0	\$45,000	0	\$45,000	0	\$0.
1603900000	CMDL Investments, LLC	1.08	\$20,212	No	N/A		\$19,000.00	per acre	0.35	\$6,650	0.25	\$4,750	0	\$0.
ssume upgrades on FDO	T property will not require property costs	•					•							
	JL Baker Life Estate	0.46	\$9,686	Yes (not affected)	N/A		\$21,000.00	per acre	0.13	\$2,730	0.09	\$1,890	0	\$0.
1605820000	Alton and Cathy Harn	0.3	\$7,920	No	N/A		\$26,000.00	per acre	0.13	\$3,380	0.09	\$2,340	0	\$0.
1605800000	Alton and Cathy Harn	0.42	\$11,808	Yes (not affected)	N/A		\$28,000.00	per acre	0.13	\$3,640	0.09	\$2,520	0	\$0.
1603980000	Alton and Cathy Harn	0.21	\$7,575	No	N/A		\$36,000.00	per acre	0.14	\$5,040	0.1	\$3,600	0	\$0.
1606850100	Carl and Kathy Adkins	2.63	\$204,607	Yes (not affected)	N/A		\$78,000.00	per acre	0.36	\$28,080	0.26	\$20,280	0	\$0.4 \$0.4 \$0.4 \$0.4
1606860100	Tampa 10 Properties, LLC	1.36	\$117,467	Yes (not affected)	N/A		\$86,000.00	per acre	0.02	\$1,720	0	\$0	0	\$0.
1606860000	Mabco Holdings, Inc.	2.88	\$317,302	Yes (not affected)	N/A		\$110,000.00	per acre	0.1	\$11,000	0	\$0	0	\$0.
1606860250	A-1 Beverage Service, Inc.	2.29	\$288,938	Yes (not affected)	N/A		\$126,000.00	per acre	0.23	\$28,980	0.11	\$13,860	0	\$0
1607270000	Solid North, LLC	0.94	\$373,700	Yes	\$400,000	12/28/2012	total purchase	total	0	\$373,700	0	\$373,700	0	\$0. \$0.
1606960000	C and S Industries	0.57	\$72,600	Yes (not affected)	N/A		\$127,000.00	per acre	0	\$0	0	\$0	0	\$0.
1604730000	Metropolitan Life Insurance	22.06	\$803,381	Yes (not affected)	N/A		\$36,000.00	per acre	0.73	\$26,280	0.51	\$18,360	0	\$0. \$0.
1604270000	1607 Property Inc.	2.61	\$210,560	Yes (not affected)	N/A		\$81,000.00	per acre	0.13	\$10,530	0.11	\$8,910	0	\$0.
				Yes (one bldg.			estimated \$45,000 per acre							
1604280000	Pamela Snead	6.45	\$290,742	affected)	\$550,000	3/5/2004	plus \$100,000 for one bldg.	total and per acre	0.38	\$117,100	0.3	\$113,500	0	\$0.
1604060000	Pamela Snead	0.54	\$58,370	No	N/A		\$108,000.00	per acre	0.17	\$18,360	0.12	\$12,960	0.12	\$12,960.
1607950000	GAC Tampa, Inc.	2.92	\$315,037	Yes (not affected)	N/A		\$108,000.00		0	\$0	0	\$0	0.1	\$10,800.
1604060100	JVS Contracting, Inc.	4.31	\$349,380	Yes (not affected)	N/A		\$81,000.00	per acre	0	\$0	0	\$0	0.12	\$9,720.
	*·				•		•	Totals =	4.34	\$690,590	3.33	\$625,870	0.38	\$37,680.0

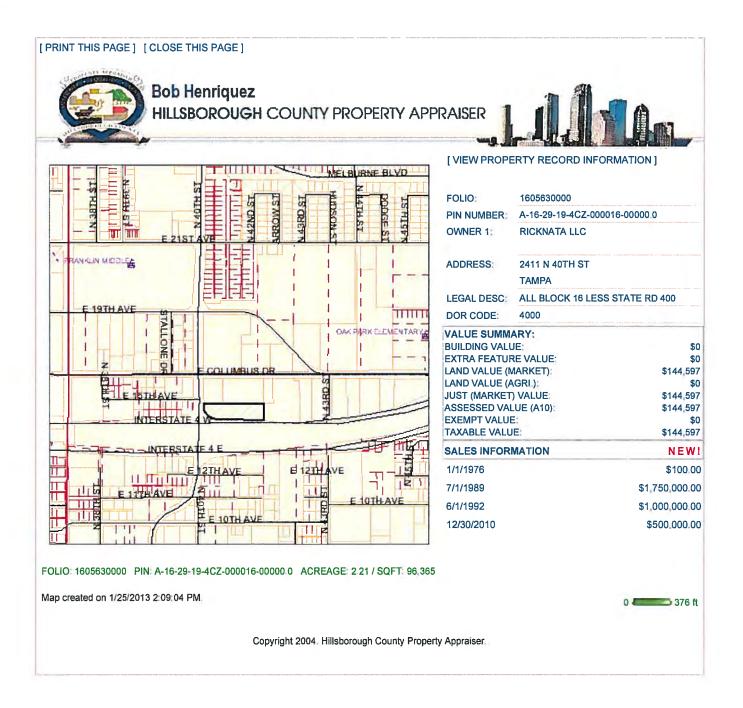
Alternative 1	
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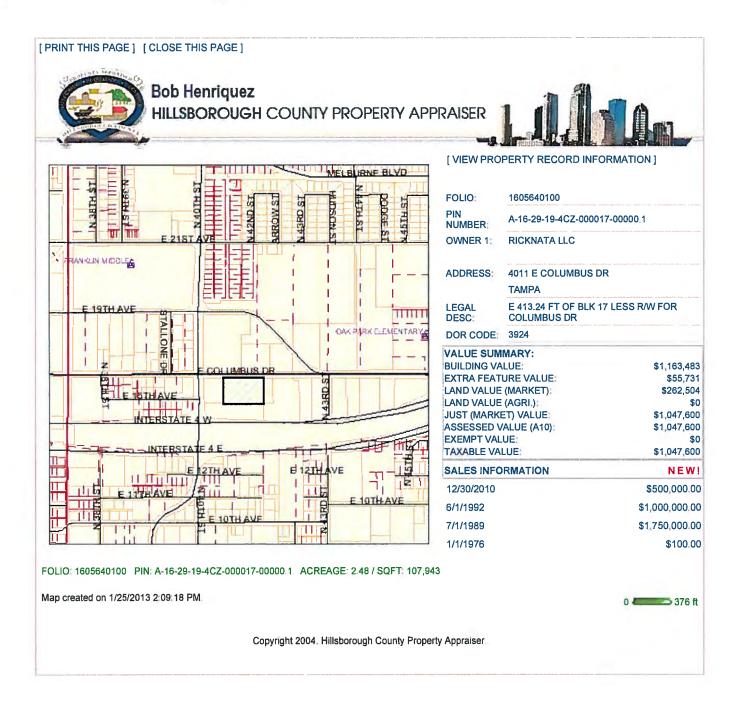
	Estimated Property Purchase Price = Estimated Administrative/Legal Costs = Estimated Total Property Costs =	\$690,590 based on prorated market land value for partial takes of property or assessed value for full property takes with buildings 50% of estimated purchase price (from sample of FDOT estimates) \$1,036,000
ernative 2		
	Estimated Property Purchase Price = Estimated Administrative/Legal Costs = Estimated Total Property Costs =	\$625,870 based on prorated market land value for partial takes of property or assessed value for full property takes with buildings \$312,935 50% of estimated purchase price (from sample of FDOT estimates)
ernative 3		
	Estimated Property Purchase Price = Estimated Administrative/Legal Costs = Estimated Total Property Costs =	\$37,680 based on prorated market land value for partial takes of property or assessed value for full property takes with buildings \$18,840 50% of estimated purchase price (from sample of FDOT estimates)

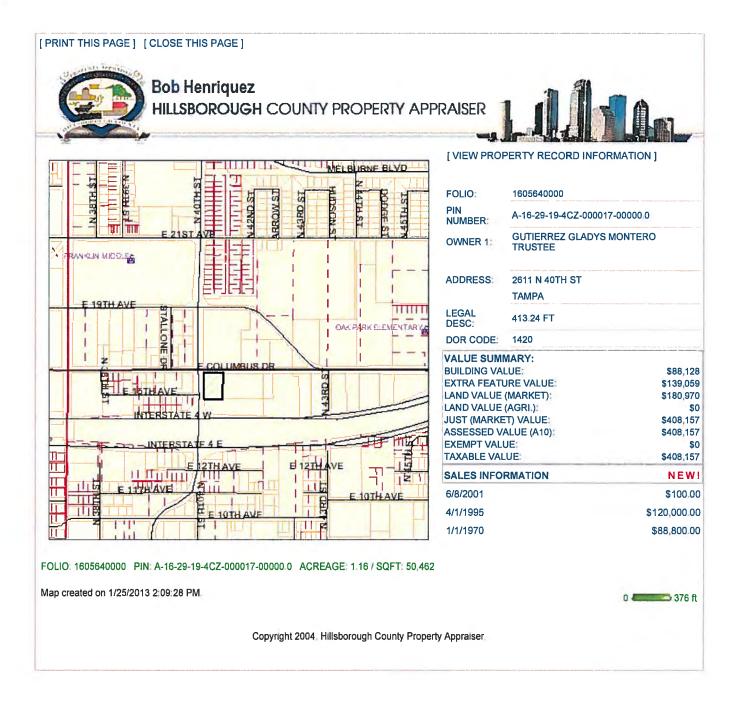


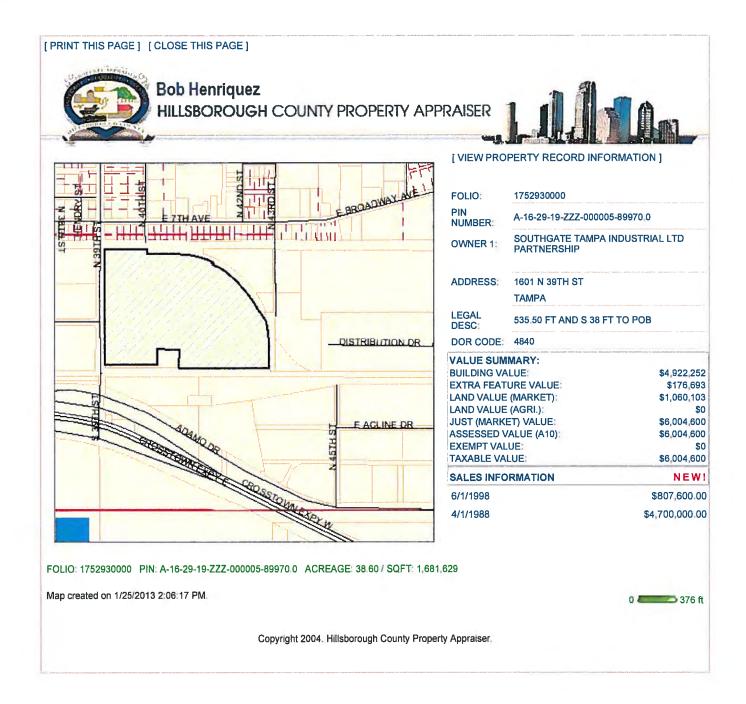


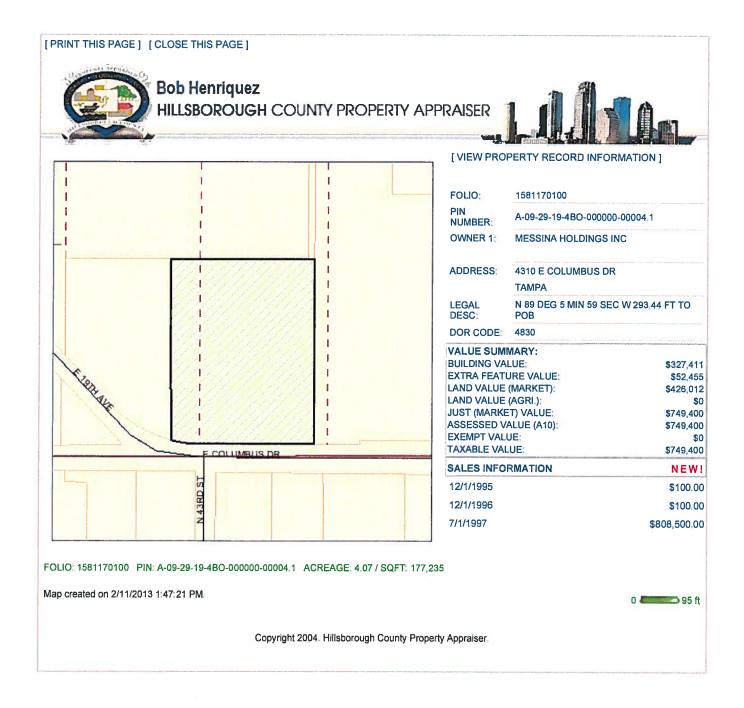


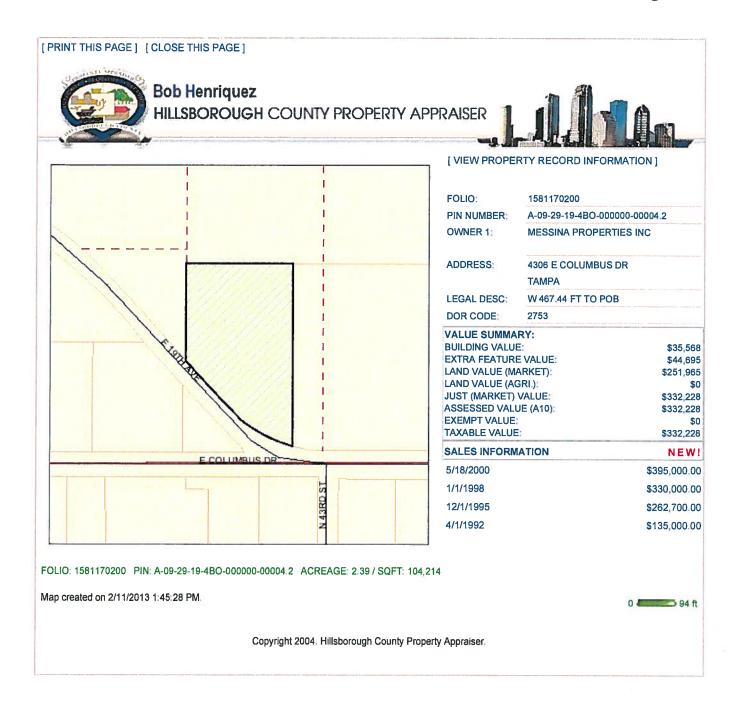


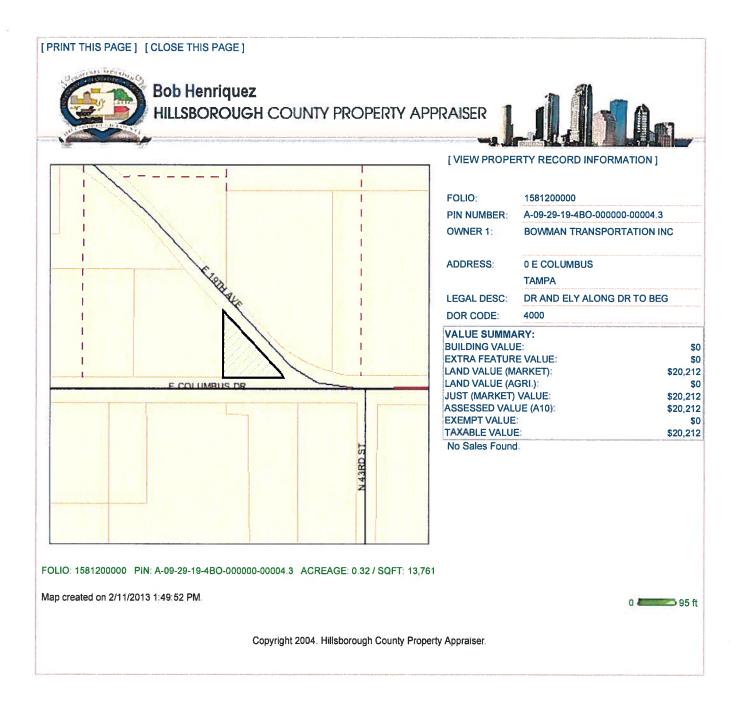


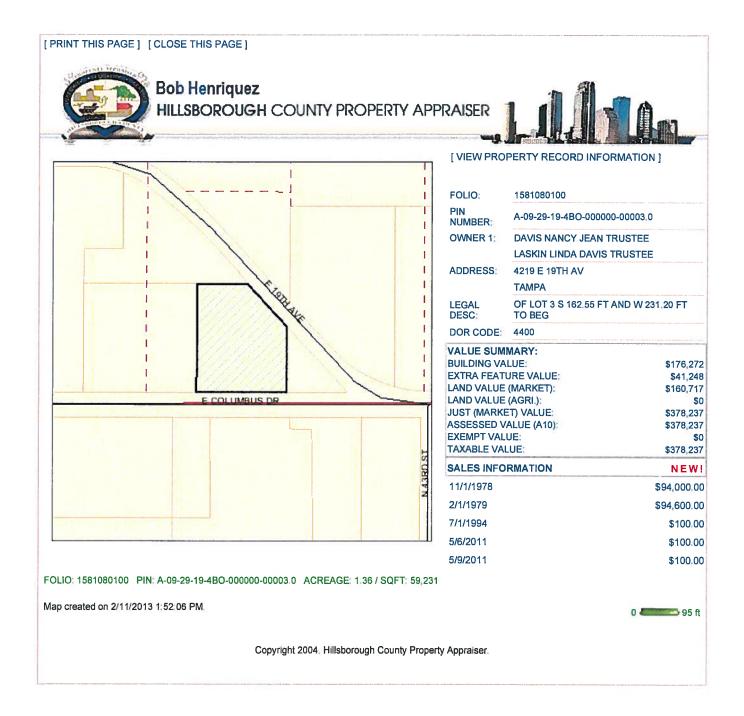




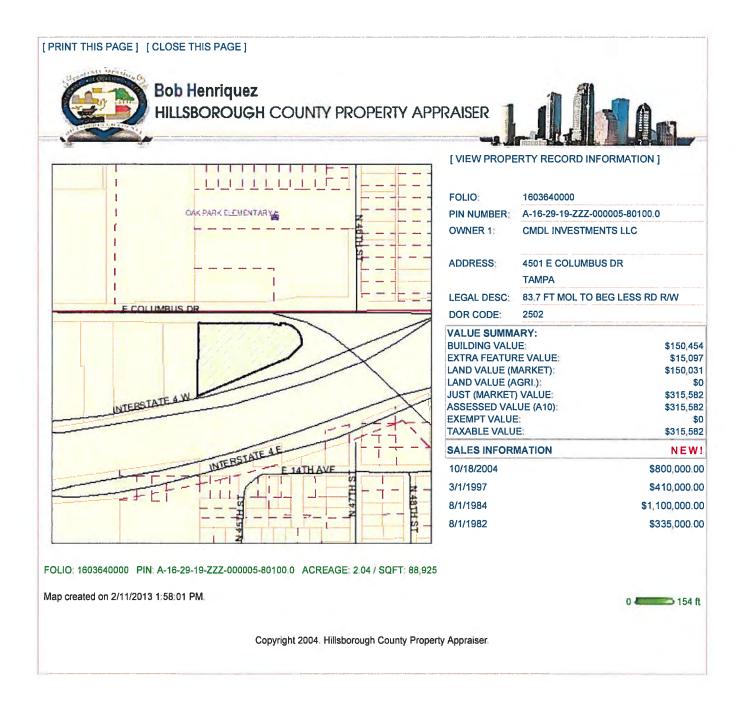


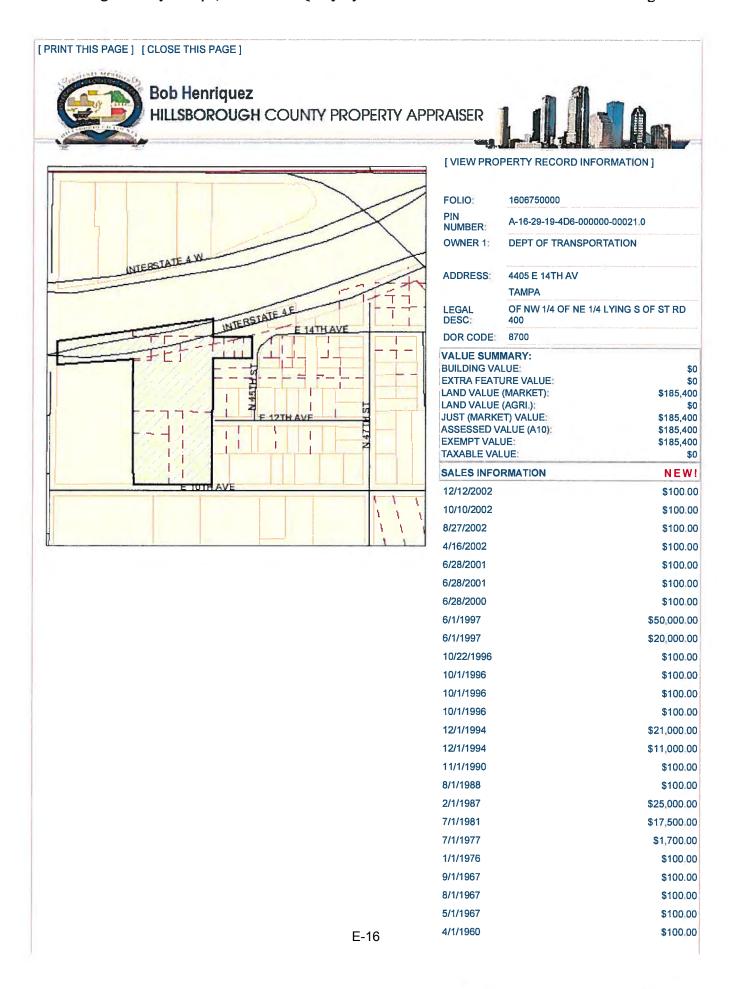


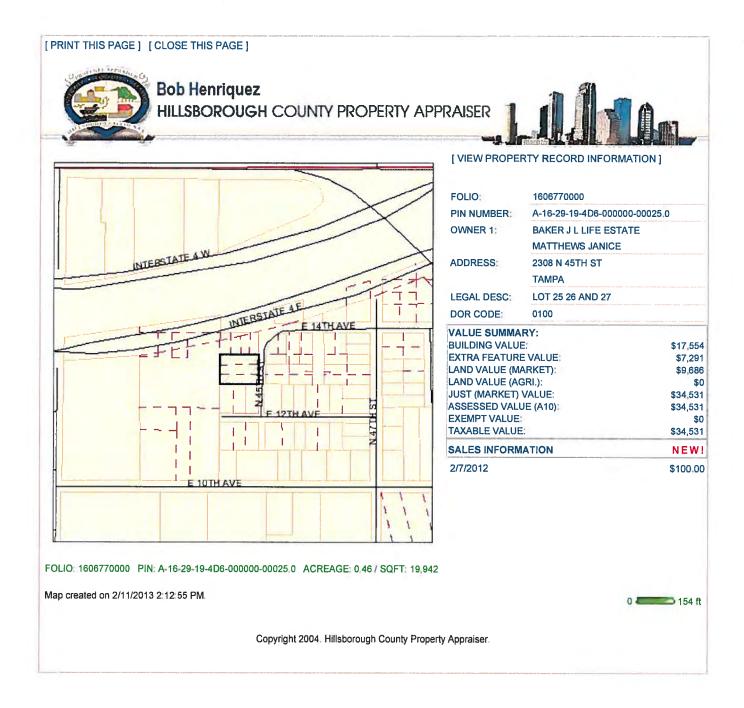


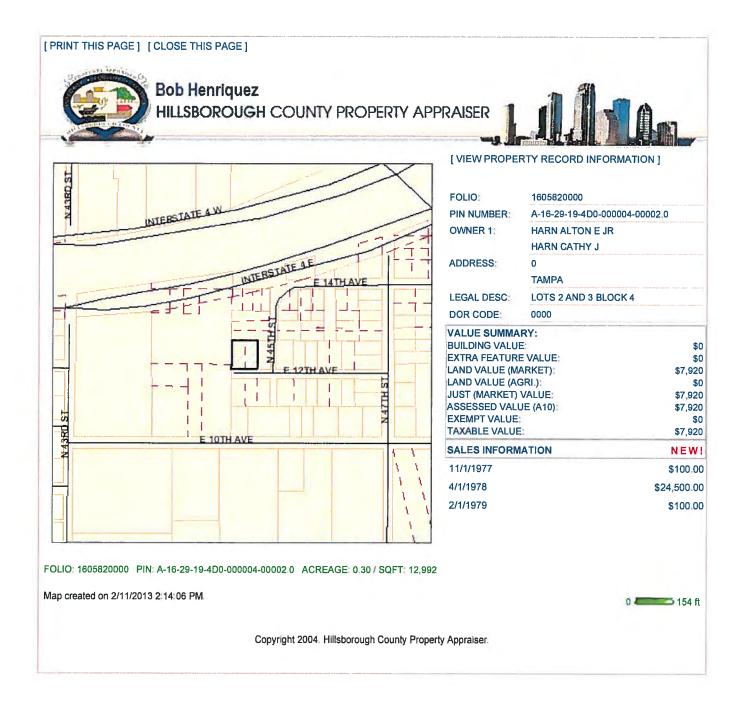


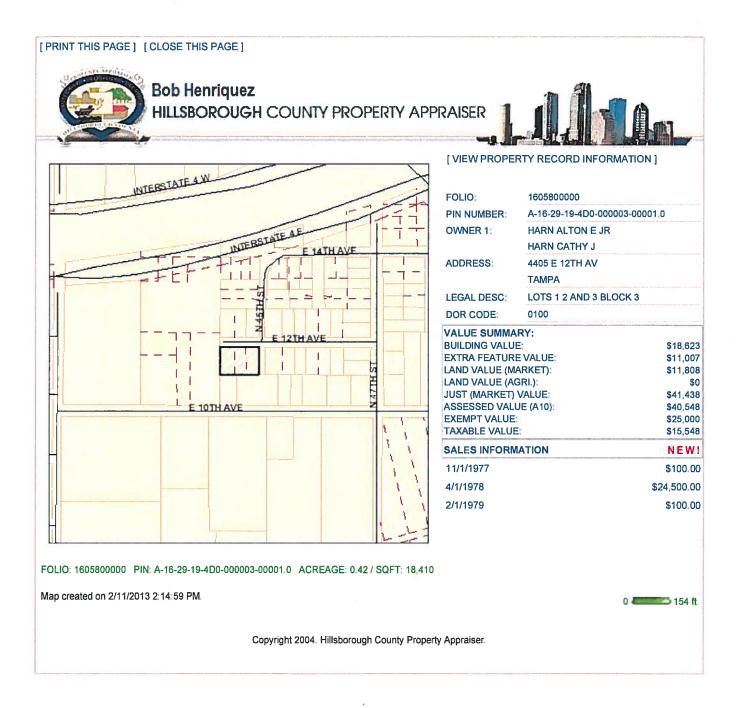


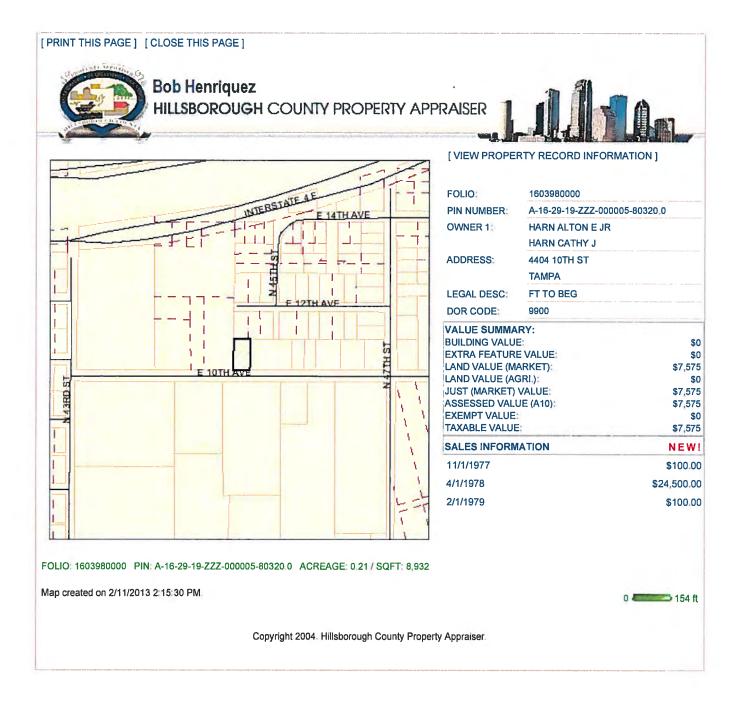


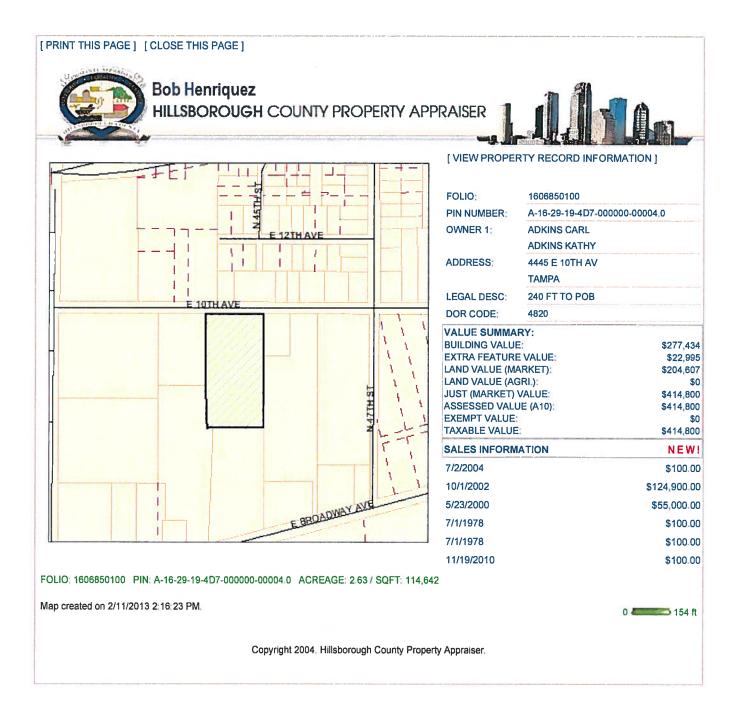


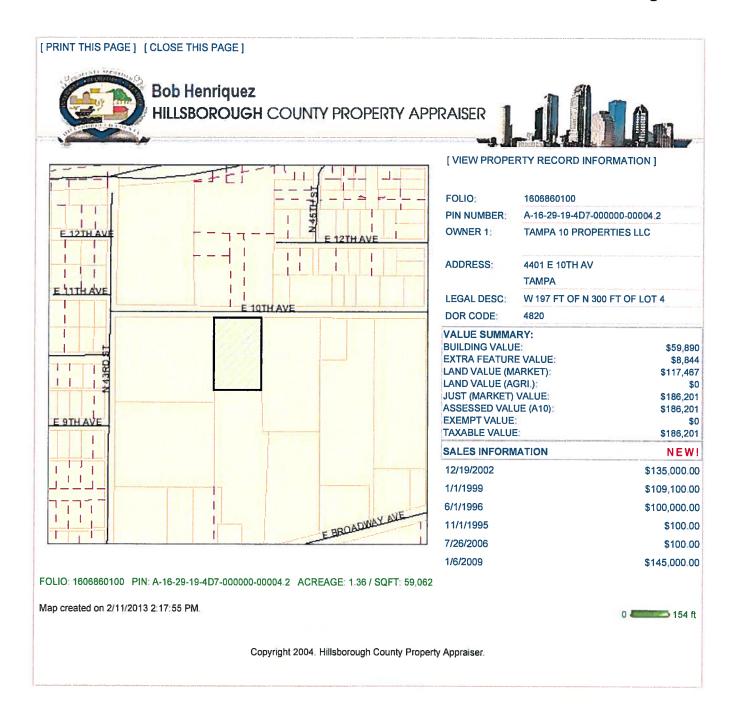


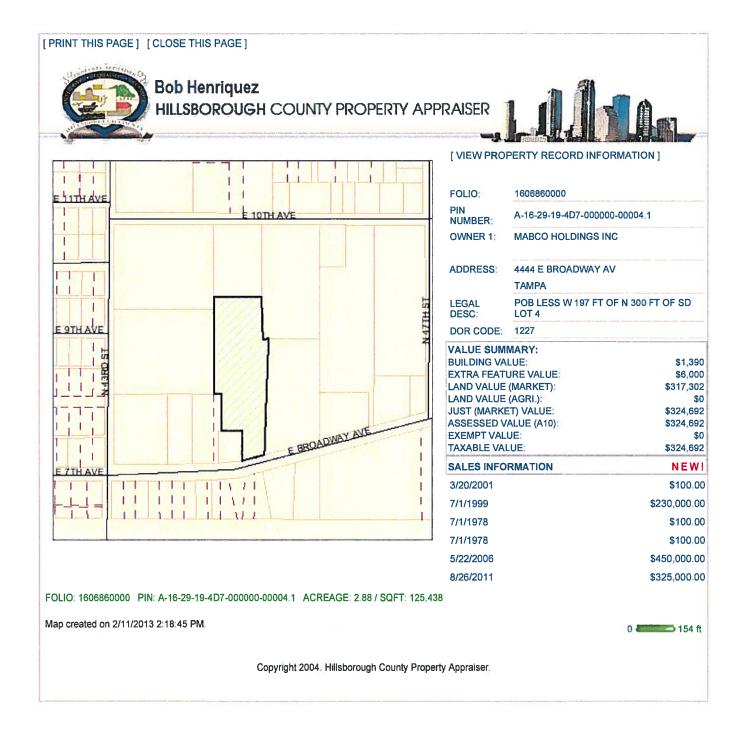


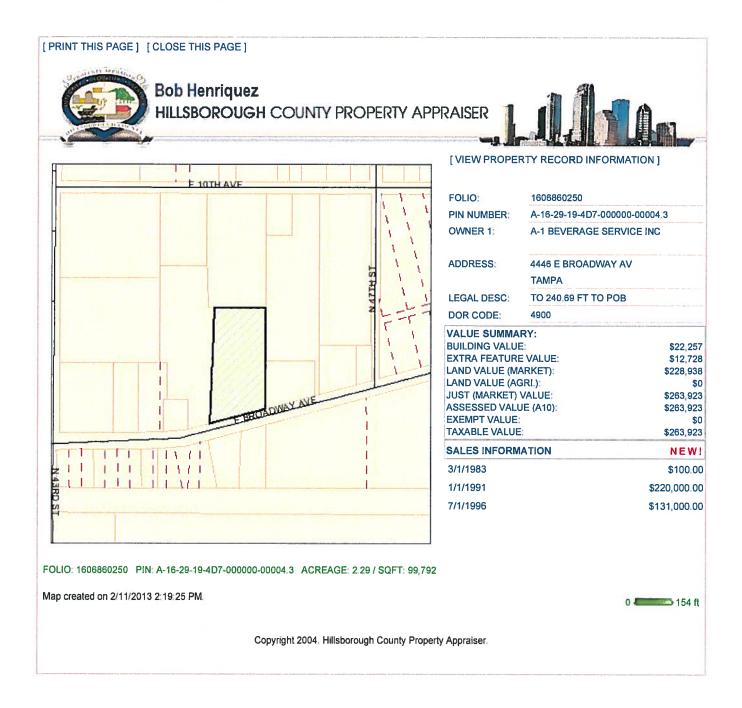


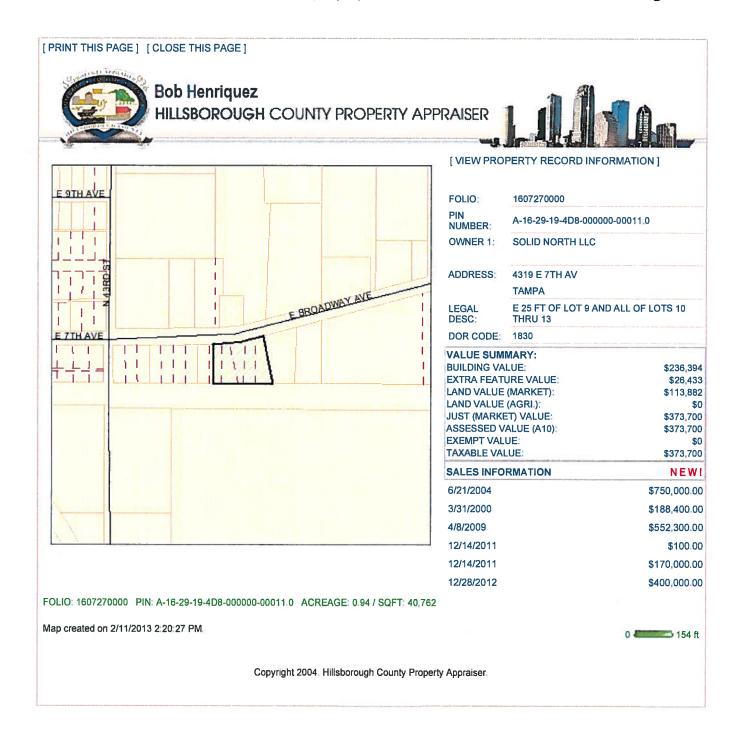


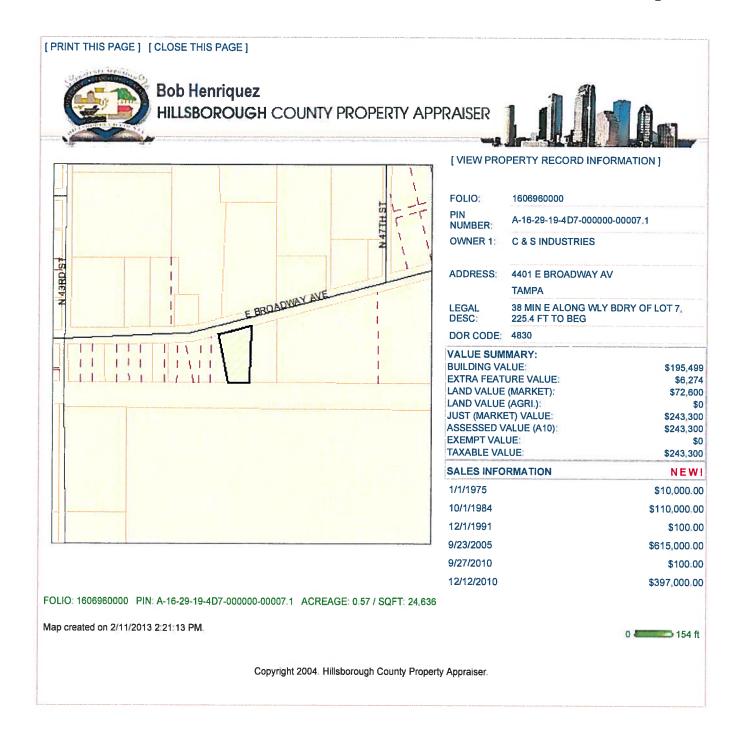


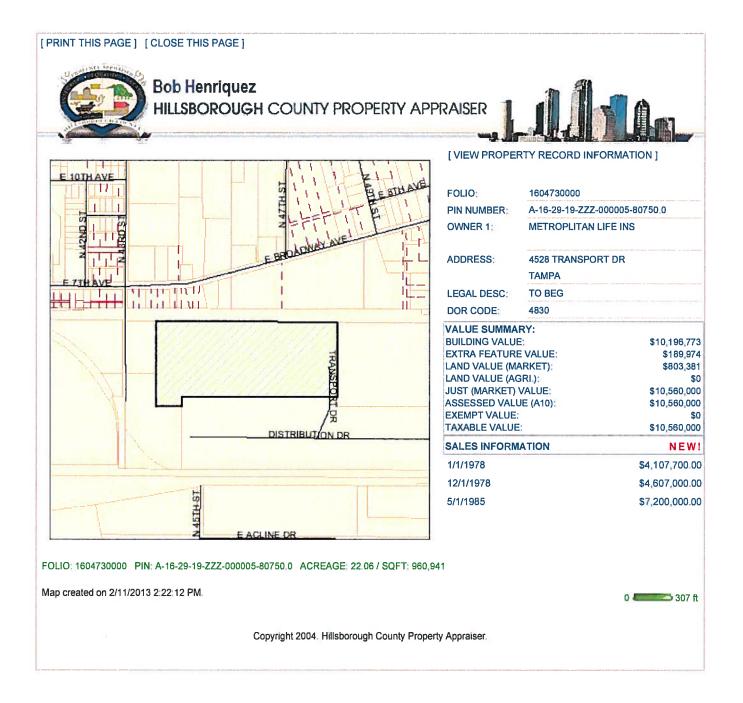


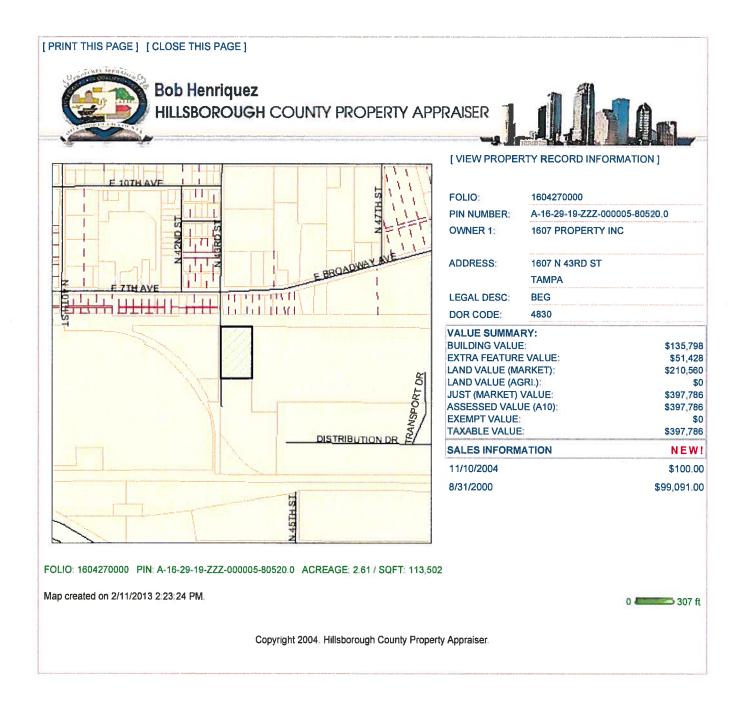


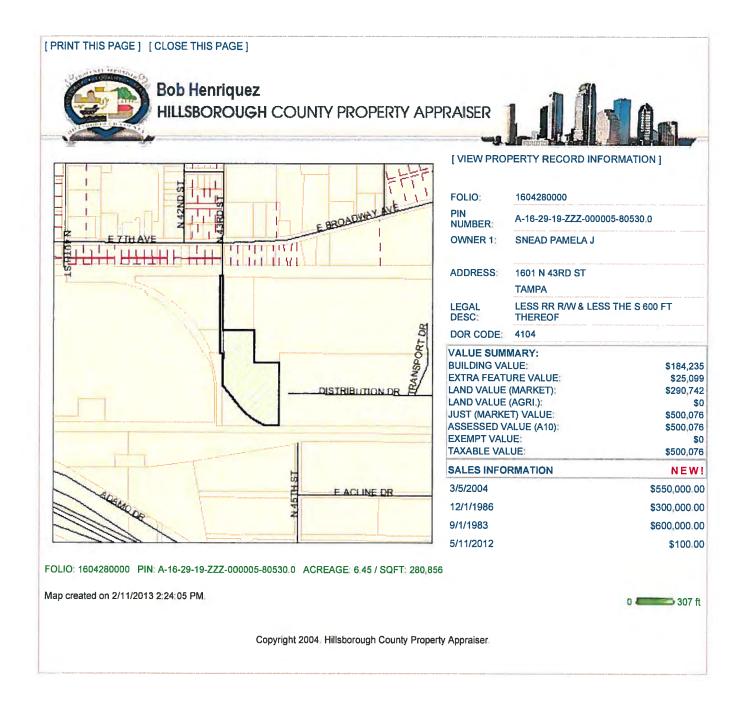


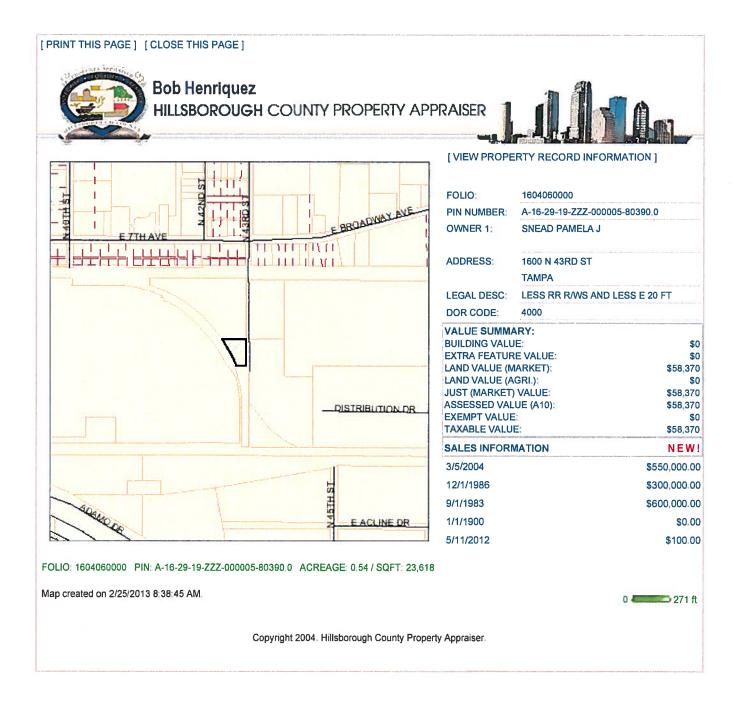


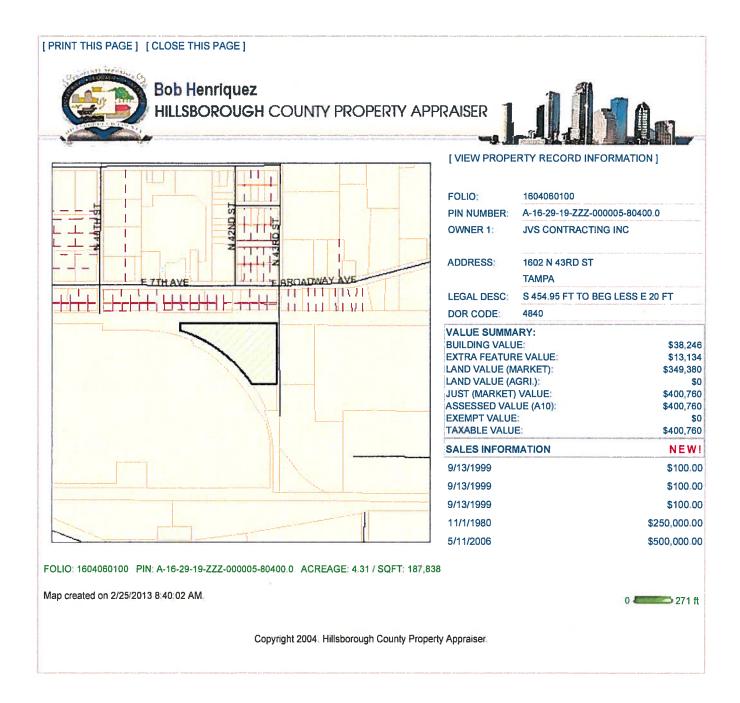


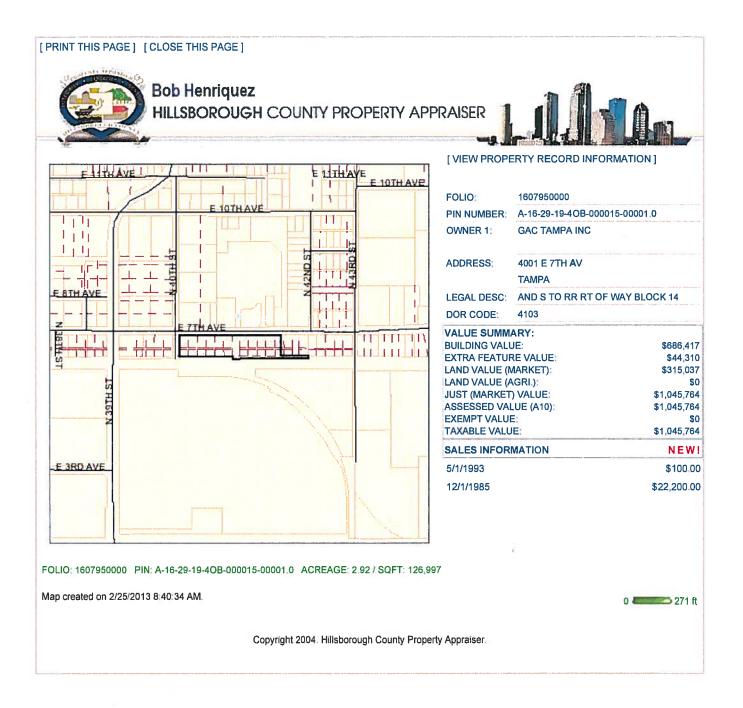












Appendix F

Utilities Information

Preliminary Utilities Information for Proposed Improvements to 43rd Street Outfall Basin

				Potential Impact	
General Location	Туре	Exact Location	Size	to Design	
		Northern lane crossing to the south to connect to system			
19th Ave.	Gravity Sanitary Sewer	on Columbus	Unknown	Low	
Columbus Ave.	Gravity Sanitary Sewer	Northern lane (east bound)	Unknown	Low	
		South side, just inside R/W. Cross drain located about 200'			
Columbus Ave.	Water Main	west of proposed Columbus crossing	12"	High	
		South side, 12' north of R/W line. Crosses Columbus about			
Columbus Ave.	Gas	340' west of proposed Columbus crossing	3"	Low	
Columbus Ave.	Telecommunications (AT&T BFO)	Near R/W on south side	N/A	Low	
Columbus Ave.	Telecommunications	North side, 6-8 feet off pavement	N/A	Low	
7th Ave.	Gravity Sanitary Sewer	Northern lane	Unknown	High	
7th Ave.	Power/Light Poles	Both sides near R/w	N/A	High	
7th Ave.	Water	18' off south R/W	12"	High	
7th Ave.	Sanitary Force Main	Westbound lane line	42"	High	
7th Ave.	Gas	North lane	3"	Low	
40th St. (north of 39th intersection)	Telecommunications (Bell System)	Btwn. Right two lanes (north bound)	N/A	Medium	
40st St. (north of 39th intersection)	Power/Light Poles	Near R/W on east side and in utility strip on west side	N/A	Medium	
40th St. (north of 39th intersection)	Gas	East side, near R/W	2"	Low	
40th St. (north of 39th intersection)	Water	West side, crossing at Columbus	16"	High	
40th St. (south of 39th intersection)	Power/Light Poles	East side, near R/W	N/A	Medium	
40th St. (south of 39th intersection)	Gravity Sanitary Sewer	West lane	Unknown	Medium	
40th St. (south of 39th intersection)	Telecomm Pedestal	SE corner of 40th and 10th (direction unknown)	N/A	Low	
40th St. (south of 39th intersection)	Water	13' off west R/W, crossing at 10th Ave.	16"	High	

Appendix G

Meeting Minutes

Principals Wayne D. Chalifoux Donaldson K. Barton, Jr. Lucius J. Cushman, Jr. Jon S. Meadows Lawrence L. Smith, Jr. William T. Stone



43RD STREET OUTFALL Kick Off Meeting

MEETING MINUTES JULY 31, 2012

Attendees:

Artie Cintron – HART, cintrona@gohart.org

Randy Stribling – HART, striblingr@gohart.org

Alex Awad – City of Tampa, DPW, alexander.awad@tampagov.net

Al Hoel – City of Tampa, Stormwater, alhoel@tampa.net

Scott Letasi – SWFWMD, scott.letasi@watermatters.org

Robin Baily – SWFWMD, robin.bailey@watermatter.org

Scott Garth – DRMP, sgarth@drmp.com

John Minton – DRMP, jminton@drmp.com

I. Co-op Funding

- Applications due October 17th
- First co-op funding meeting is August 17th
- Money would be available in October 2013 (FY 14)
- We could do co-op funding for design cost only future funding for construction cost

II. HART Survey – NAVD 88 by Charlotte Survey

- Some preoperty to the South was bermed off by a private property owner.
- Scott Garth to request survey be done in 1988

III. Models

- Any adjacent models available for boundary conditions
- No need to model anything under 24"
- Scott Letasi check city GIS inventory
 - 24" pipes and above
- Interconnetivity at Acline Mike Miller
- DRMP to look for Government owned lands

IV. Flooding

- Did not Occur during Tropical Storm Debbie (June 24-25, 2012).
- Storm has to be a shorter duration
- Scott Garth to contact Megan at FDOT for complaints.
 - Wait until after existing conditions for FDOT meeting
- Robin Bailey can provide SWFWMD complaint database

V. Funding

- Funding is 75% city; 25% HART

CC: Attendees File

3816 W. Linebaugh Ave. Suite 400 Tampa, Florida 33618 Phone: 813.265.9800 Fax: 813.265.9822

Principals Wayne D. Chalifoux Donaldson K. Barton, Jr. Lucius J. Cushman, Jr. Jon S. Meadows Lawrence L. Smith, Jr. William T. Stone



43RD STREET OUTFALL Progress Meeting

MEETING MINUTES NOVEMBER 16, 2012

Attendees:

Alex Awad – City of Tampa, DPW John Early – City of Tampa Scott Garth – DRMP Ken Kniel - DRMP

- I. SWFWMD application was for FY 14 (July is the beginning of FY 14), so time is OK
- **II.** In August 2013, city will need to apply for FY 15 grants
- **III.** City is OK with SCS method (CN) and FDOT rainfall distributions if SWFWMD is.
- **IV.** Identify properties needed and viable properties
- V. Invite Robin Bailey and Scott Letasi
- **VI.** Ask Dallas if any water/sewer GIS
- VII. Contact Yvette Bowman or Mike Miller if we want to use city actual unit cost, but using FDOT historical unit cost is OK
 - Use property appraised value for property cost
- VIII. Contact Jimmy Cook or Barbara Lynch on CDC on North Boulevard
 - Give them H-15 (Atlas Page) and they can research easements for us.

CC: Attendees File

Principals Wayne D. Chalifoux Donaldson K. Barton, Jr. Lucius J. Cushman, Jr. Jon S. Meadows Lawrence L. Smith, Jr. William T. Stone



43RD STREET OUTFALL STUDY

Project ST510C Final BODR Meeting City of Tampa Office – 6th Floor Conference Room

MEETING MINUTES MARCH 1, 2013

Attendees:

Alex Awad, City of Tampa John Early, City of Tampa Randy Stribling, HART Lynda Crescentini, HART Scott Garth, DRMP John Minton, DRMP Ken Kniel, DRMP

The final BODR meeting was held at the City of Tampa Office to discuss the 43rd Street Outfall Study. Below is a summary of the items discussed.

- John Minton went over alternatives.
 - o Alternative 1: Conveyance upgrades
 - o Alternative 2: 5 acre site 4-sale \$1.8M (Alex)
 - 3.3 acre site vacant \$0.50M active hotel willing to sell
 - o Alternative 3: By-Pass
 - New Outfall down 40th Street
 - Minimal Upgrades to existing channel
- Question 1: Is South pond a viable option (i.e. is it a wetland)? Answer: Some wetlands associated with ditching.
- Question 2: Alex asked if we could consider using all three parcels up North.
 - Lynda asked if we could check with FDOT for any surplus parcels for I-4 (Bill Scott – FDOT).
- \$1.5M FY 2014 for property acquisition...\$50K each to design (\$100K)
- Geotech will be needed for ponds and contamination potential
- Lynda Agreed to initate meeting with FDOT
 - Scott to determine FDOT PM for 39th Street RRR for Lynda
- Application due date for FY 2015 would be October 2014

Principals Wayne D. Chalifoux Donaldson K. Barton, Jr. Lucius J. Cushman, Jr. Jon S. Meadows Lawrence L. Smith, Jr. William T. Stone



- Alex requested that DRMP do another analysis to use the 7.1 acre (South side of Columbus Drive) + 5 acre (North side of Columbus Drive) to reduce size of box culvert down 40th Street to pipe.
 - o Could the 30" pipe under CSX remain as is?

ACTION ITEMS:

- 1. DRMP (Scott Garth) to determine FDOT PM for 39th Street RRR
- 2. DRMP (Scott Garth) to contact Bill Scott at FDOT for any surplus R/W
- 3. HART (Lynda Cresentini) to set up FDOT meeting

cc: File Attendees

Appendix H

Alternative 4

Development of Fourth Alternative

The City has reviewed the draft of the report and conducted preliminary research on potential pond sites considered in the study. With the availability of properties in the study area, the City requested for DRMP to consider an additional alternative that relies more heavily on property acquisition in the vicinity of the flooding problem area along Columbus Drive. The addition of this fourth alternative is included in the final report as Appendix H. This fourth alternative is intended to resemble Alternative 3 with a diversion system along 40th Street, that is reduced in size from a box culvert to a pipe. The reduced size of the diversion system is beneficial considering close proximity to utilities and bore/jack requirements under the railroad tracks.

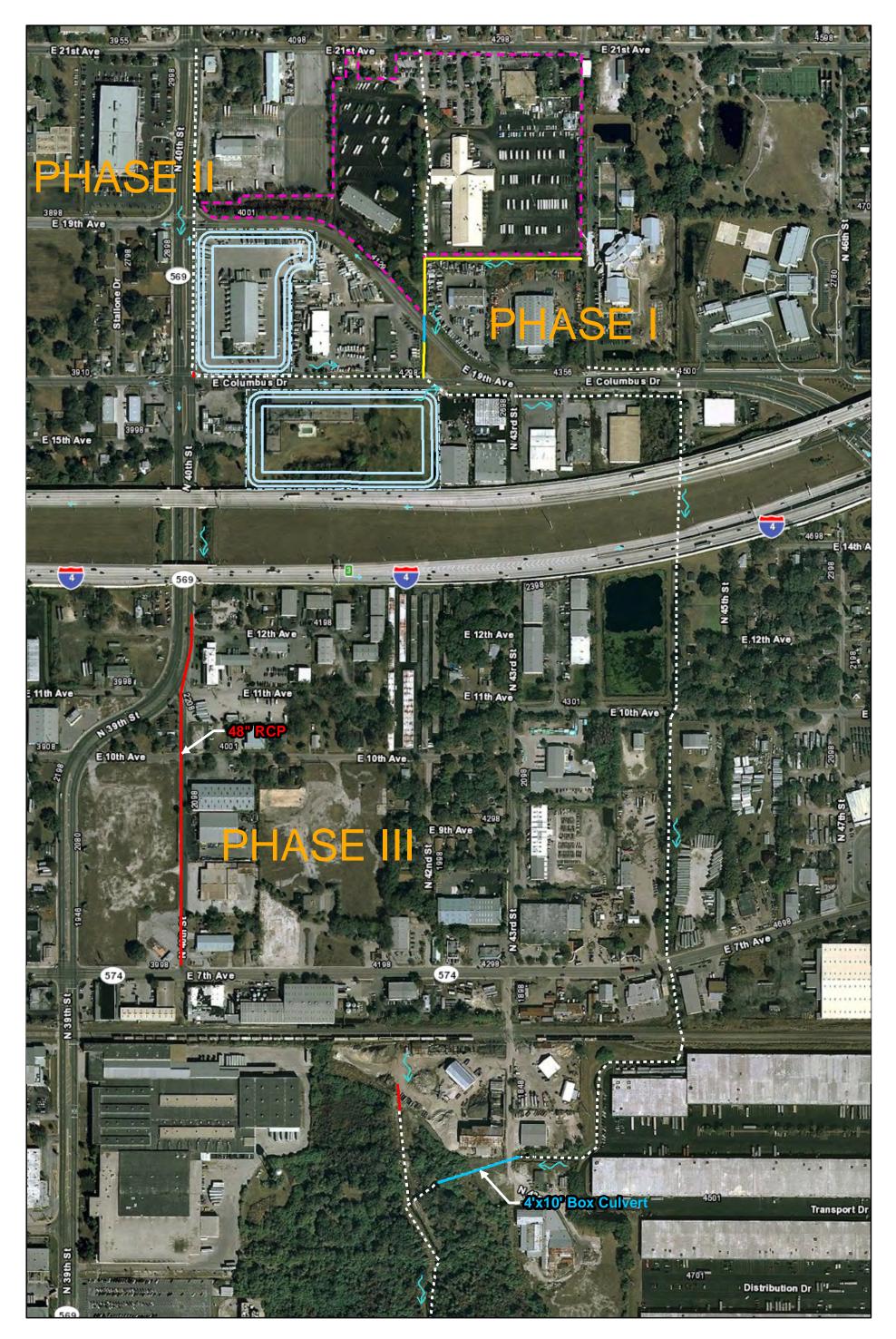
Alternative 4: Diversion System with two (2) North Ponds

- Diversion system along 40th Street with **48-inch pipe**, distance of 3,900 linear feet.
- Acquisition of additional **0.4 acres** (affecting 4 properties) of right-of-way or easements for culvert work.
- Culvert crossing upgrades with single box culverts/pipes at **two** (2) roadway crossings and **one** (1) railroad crossing.
- Construction of two stormwater ponds on either side of Columbus Drive sized at **7.1 acres** and **5.7 acres** that will provide flood storage, attenuation, and stormwater treatment (two North Ponds).

Alternative 4 differs from Alternative 3 in that the diversion system size has been substantially reduced in size from a 4' x 6' box culvert to a 48-inch pipe, and stormwater attenuation is sufficient in the two (2) north ponds to eliminate the need for the south pond. Characteristics of the preliminary design for the additional north pond are described below with the primary design function in the heading (elevations in ft, NAVD88).

North Pond (5.7 Ac) – stormwater attenuation/flood storage

- On-line Wet Detention
- Property acreage = 5.7 acres (North side of Columbus Drive)
- Berm Elevation = 29.0
- Control Elevation = 25.0
- Bottom Elevation = 16.5 (max. depth of 8.5 feet)
- Permanent Pool Volume = 28.2 ac-ft





43rd Street Outfall Basin Study **Alternative 4 - Diversion System**with 2 North Ponds

As indicated in the table of peak stages below, Alternative 4 provides similar levels of flood protection as Alternative 3 at the critical areas of the study.

Table H-1 – Peak Stages at Select Nodes in Focus Area for Proposed Conditions

Junction Name	Location			tage (ft, N 'ear, 8-Ho	Critical Elevation				
		Existing	Alt. 1	Alt. 2	Alt. 3	Alt. 4			
	HART Facility								
							26.9 low pavement elevation in		
NA0655	Southeast corner of HART facility	28.32	26.85	26.88	26.89	26.80	parking area		
	HART facility outfall ditch, north side								
NA0605	of 19th Avenue crossing	28.44	26.82	26.84	26.84	26.79	27.8 low edge of pavement		
	43rd Street Outfall Ditch								
NA0210 Upstream side of I-4 crossing		27.86	25.92	26.03	26.21	26.20	28.0 top of ditch bank		
NA0215	Downstream side of I-4 crossing	27.61	24.81	25.70	26.12	26.10	28.0 top of ditch bank		
NA1005	Upstream side of 10th Avenue crossing	27.52	24.38	25.39	26.05	26.03	26.0 low edge of pavement		
NA0230	Upstream side of 7th Avenue crossing	25.93	21.78	23.13	25.61	25.56	30.1 top of ditch bank		
NA0240	Upstream side of railroad crossings	25.02	19.39	20.86	24.56	24.50	29.1 low point of rail bed		
NA0255	Upstream side of 43rd Street crossing	18.84	16.50	16.79	17.12	14.82	18.5 low top of ditch bank		

In the table included below, peak flows are compared in the channel downstream of the confluence with the proposed diversion system. For Alternative 4, proposed peak flows remain below existing conditions providing assurance of no adverse impacts downstream. Additionally, the existing and proposed peak flows are compared as required by SWFWMD.

Table H-2 – Peak Flows Comparison for Proposed Conditions

		Peak Flow (CFS)										
Link Nam	e Location	5-Year, 8-Hour					25-Year, 24-Hour					
		Existing	Alt. 1	Alt. 2	Alt. 3	Alt. 4	Existing	Alt. 1	Alt. 2	Alt. 3	Alt. 4	
	43rd St Outfall											
RA0280	Ditch, South of	399	353	317	319	396	491	484	448	460	485	
	Railroad											

Discussion and Comparison of Alternatives

Alternative 4 involves acquisition of an additional property with an existing building and road frontages. If property acquisitions for Alternative 4 go favorably, the overall project cost would be significantly reduced by downsizing the diversion system (see summary of estimated alternative costs below). Additionally, other benefits with a downsized diversion system are envisioned including easier utility conflict resolution, lower difficulty and faster construction, lower maintenance of traffic demands, and less disruption to property owners (especially at CSX railroad crossing).

Table H-3 – Summary of Estimated Project Costs for Alternatives

Alternative	Estimated Project Cost		
1: Conveyance Upgrades with South Pond	\$	9,110,000	
2: Conveyance Upgrades with North and South Ponds	\$	9,149,000	
3: Diversion System with North and South Ponds	\$	8,963,000	
4: Diversion System with Two (2) North Ponds	\$	7,123,000	

Recommendation and Phasing

The amended recommendation for this study is to proceed with feasibility investigations into purchasing the three potential pond sites included in both Alternatives 3 and 4 as listed below:

- 1. North Pond (7.1 Ac) south of Columbus Drive w/ two-story building
- 2. North Pond (5.7 Ac) north of Columbus Drive w/ warehouse building
- 3. South Pond (9.7 Ac) west of 43rd Street crossing

At this time, Alternative 4 is the recommendation of this study due to considerable construction and design savings. However, any combination of two out of these three properties will satisfy the needs for pond construction for this project.

Included below is a schedule of possible phasing for Alternative 4.

Table H-4 – Phasing of Recommendation

	Esti	mated Property	Estimated Design and		Total Estimated	
Phases		Costs	Construction Costs		Phase Cost	
1: Construct North Pond (7.1 Ac) and connection						
to HART	\$	1,500,000	\$	1,266,000	\$	2,766,000
2: Construct North Pond (5.7 Ac)	\$	1,500,000	\$	654,000	\$	2,154,000
3: Construct diversion system along 40th Street						
and 43rd Street box culvert	\$	57,000	\$	2,146,000	\$	2,203,000
				Total -	Ċ	7 122 000

Total = \$ 7,123,000

Included below is a breakdown of the estimated construction, design, and property costs for Alternative 4.

Preliminary Project Cost Estimate for Alternative 4

tem Number	Description	Unit Cost	Units	Quantity	Total Cost
1	Clearing and Grubbing	\$7,500.00	AC	27.2	\$204,000
2	Structure with 10-foot Diversion Weir	\$10,000.00	EA	0	9
3	Modify Drainage Structure (connect pipe)	\$1,500.00	EA	2	\$3,000
4	Manhole Riser (for access to box culvert)	\$3,000.00	EA	12	\$36,000
5	Concrete for Endwall	\$670.00	CY	34.8	\$23,329
6	Building Demolition (with wrecking ball)	\$2.00	SF	84,200	\$168,40
7	Reinforced Concrete Pipe (RCP), 48-inch	\$130.00	LF	3,780	\$491,40
8	Box Culvert, 3' x 5'	\$421.00	LF	90	\$37,890
9	Box Culvert, 4' x 6'	\$526.00	LF	0	
10	Box Culvert, 4' x 10'	\$736.00	LF	335	\$246,560
11	Box Culvert, 4' x 11'	\$789.00	LF	67	\$52,863
12	Channel Excavation	\$6.80	CY	1,867	\$12,69
13	Pond Excavation	\$4.00	CY	158,864	\$635,450
14	Pond Berm Embankment	\$5.00	CY	76	\$380
15	Performance Turf (Sod)	\$1.60	SY	36,683	\$58,693
16	Chain-link Fence (6')	\$13.00	LF	4,450	\$57,850
17	Concrete Sidewalk (Driveway), 6-inch Thick	\$38.00	SY	981	\$37,27
18	Concrete Sidewalk, 4-inch Thick	\$29.00	SY	1,190	\$34,510
19	Roadway Open Cut Restoration	\$57.00	SY	375	\$21,37
20	Roadway Lane Restoration	\$43.00	SY	6,714	\$288,70
	Maintenance of Traffic (12%)				\$289,24
	Mobilization & Demobilization (10%)				\$269,96
	Contingency Cost (20%)				\$593,91
	Construction Cost Sub-total				\$3,563,508
	Construction Engineering Inspection and Testing (5%)				\$178,17
	Engineering Design and Permitting (7%)				\$249,44
	Railroad Compliance and Permitting	\$75,000.00	LS	1	\$75,00
	Property Costs for Minor Conv. Upgrades and two (2) Nor	th Ponds (see se	parate tab	ulation)	\$3,057,00
	Estimated Project Costs, Alternative 4				\$7,123,000

Cost Estimate Notes:

- 1) Unit Costs are based on FDOT Item Average Statewide Unit Costs (2011), except as noted below.
- 2) Building Demolition unit prices are from "Get-A-Quote.net".
- 3) Maintenance of traffic includes permanent striping and signing.
- 4) Roadway Open Cut Restoration based on Type B Stabilization (Pay Item No. 160-4) at \$3.10 /sy, Optional Base Group 9 (Pay Item No. 285-709) at \$12.30/sy, and 3 inches of Superpave Asphaltic Concrete (Pay Item No. 334-1-13) at \$13.00/sy with 50% contingency for milling and additional overlay pavement for lane reconstruction and 100% continency for open cut restoration.